Integration of the Equation of Motion

Step-by-step Numerical Procedures

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Outline

Previous Methods

Both the Duhamel integral and the Fourier transform methods lie on on the principle of superposition, i.e., superposition of the responses

- to a succession of infinitesimal impulses, using a convolution (Duhamel) integral, when operating in time domain
- to an infinity of infinitesimal harmonic components, using the frequency response function, when operating in frequency domain.

The principle of superposition implies *linearity*, but this assumption is often invalid, e.g., a severe earthquake is expected to induce inelastic deformation in a code-designed structure.

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Step-by-step Methods

The so-called step-by-step methods avoid any use of superposition, and are then well suited to be extended to treat the case of non linear structural behaviour.
The state of a dynamical system is described in terms of a set of parameters, including displacements and velocity.
For a linear system, the initial and final states are simply the displacement and the velocity (the so called *state vector*), while for a non linear system the state vector must include other information, e.g. the current tangent stiffness, the cumulated plastic deformations, etc.

Step-by-step Methods, 2

Given an initial system state, in step-by-step methods we divide the time in *steps* of known, short duration h_i (usually $\mathrm{h}_\mathrm{i} = \mathrm{h}$, a constant) and from the initial system state at the beginning of each step we compute the final system state at the end of each step, to be used as the initial state in the subsequent step.

Operating independently the analysis for each time step, there are no requirements for superposition, and non linear behaviour can be considered (or maybe approximated by a *local* linear model) assuming that the structural properties remain constant during each time step.

If the achieved approximations are not good enough, usually better approximation can be obtained reducing the time step.

Advantages of s-b-s methods

The step-by-step methods are very general, they can deal with every kind of non-linearity, e.g. variation in mass or damping or variation in geometry, and not only with stiffness non-linearity.
The step-by-step methods are very efficient, and are usually preferred, also for linear systems, with respect to the Duhamel integral method.
The step-by-step methods can be easily extended to systems with many degrees of freedom, simply using matrices and vectors in place of scalar quantities.

Disadvantages of s-b-s methods

The step-by-step methods are approximate numerical methods, that can give only an approximation of true response. The causes of error are

- roundoff, resulting from using too few digits in calculations,
- truncation, using too few terms in series expressions of quantities,
- instability, resulting from amplification of other errors from one step in the computations in the other steps, usually dependent on the time step duration.

Errors may be classified as

- phase shifts or change in frequency of the response,
- artificial damping, the numerical procedure removes or adds energy to the dynamic system.

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Piecewise exact method

- We use the exact solution of the equation of motion for a system excited by a linearly varying force, so the source of all errors lies in the piecewise linearisation of the force function and in the approximation due to a local linear model.
- We will see that an appropriate time step can be decided in terms of the number of points required to accurately describe either the force or the response function.

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Piecewise exact method

For a generic time step of duration h, consider

- $\{x_0, \dot{x}_0\}$ the initial state vector,
- p_0 and p_1, the values of $p(t)$ at the start and the end of the integration step,
- the linearised force

$$p(\tau) = p_0 + \alpha\tau,\ 0 \leqslant \tau \leqslant h,\ \alpha = (p(h) - p(0))/h,$$

- the forced response

$$x = e^{-\zeta\omega\tau}(A\cos(\omega_d\tau)+B\sin(\omega_d\tau))+(\alpha k\tau+kp_0-\alpha c)/k^2,$$

where k and c are the stiffness and damping of the SDOF system.

Piecewise exact method

Evaluating the response x and the velocity $\dot{x}$ for $\tau = 0$ and equating to $\{x_0, \dot{x}_0\}$, writing $\Delta_{st} = p(0)/k$ and $\delta\Delta_{st} = (p(h) - p(0))/k$, one can find A and B

$$A = \frac{\dot{x}_0 + \zeta\omega B - \frac{\delta\Delta_{st}}{h}}{\omega_d}$$

$$B = x_0 + \frac{2\zeta}{\omega}\frac{\delta\Delta_{st}}{h} - \Delta_{st}$$

substituting and evaluating for $\tau = h$ one finds the state vector at the end of the step.

Piecewise exact method

With $\mathcal{S}_{\zeta,h} = \sin(\omega_d h)\exp(-\zeta\omega h)$ and $\mathcal{C}_{\zeta,h} = \cos(\omega_d h)\exp(-\zeta\omega h)$ and the previous definitions of Δ_{st} and $\delta\Delta_{st}$, finally we can write

$$x(h) = A\,\mathcal{S}_{\zeta,h} + B\,\mathcal{C}_{\zeta,h} + (\Delta_{st} + \delta\Delta_{st}) - \frac{2\zeta}{\omega}\frac{\delta\Delta_{st}}{h}$$

$$\dot{x}(h) = A(\omega_d\mathcal{C}_{\zeta,h} - \zeta\omega\mathcal{S}_{\zeta,h}) - B(\zeta\omega\mathcal{C}_{\zeta,h} + \omega_d\mathcal{S}_{\zeta,h}) + \frac{\delta\Delta_{st}}{h}$$

where

$$B = x_0 + \frac{2\zeta}{\omega}\frac{\delta\Delta_{st}}{h} - \Delta_{st}$$

$$A = \frac{\dot{x}_0 + \zeta\omega B - \frac{\delta\Delta_{st}}{h}}{\omega_d}$$

Example

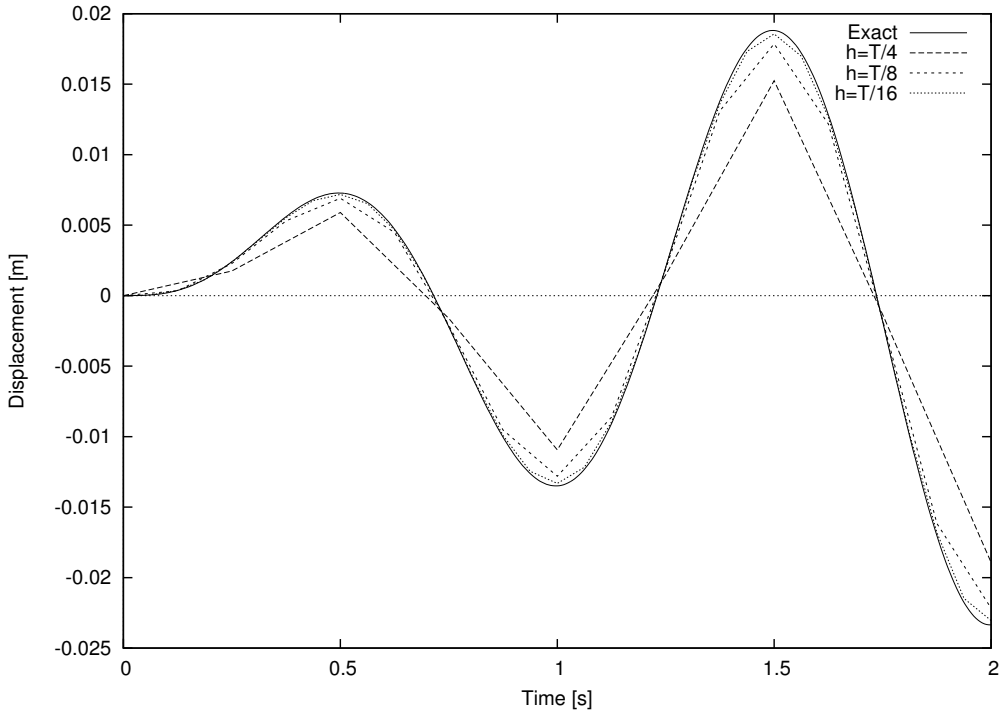
$m=1000kg$,

$k=4\pi^2\,1000N/m$,

$\omega=2\pi$,

$\zeta=0.05$,

$p(t)=4\pi^2 5N \sin(2\pi t)$



It is apparent that you have a very good approximation when the linearised loading is a very good approximation of the input function, let's say $h \leqslant T/10$.

Central differences

To derive the Central Differences Method, we write the eq. of motion at time $\tau = 0$ and find the initial acceleration,

$$m\ddot{x}_0 + c\dot{x}_0 + kx_0 = p_0 \Rightarrow \ddot{x}_0 = \frac{1}{m}(p_0 - c\dot{x}_0 - kx_0)$$

On the other hand, the initial acceleration can be expressed in terms of finite differences,

$$\ddot{x}_0 = \frac{x_1 - 2x_0 + x_{-1}}{h^2} = \frac{1}{m}(p_0 - c\dot{x}_0 - kx_0)$$

solving for x_1

$$x_1 = 2x_0 - x_{-1} + \frac{h^2}{m}(p_0 - c\dot{x}_0 - kx_0)$$

Central differences

We have an expression for x_1, the displacement at the end of the step,

$$x_1 = 2x_0 - x_{-1} + \frac{h^2}{m}(p_0 - c\dot{x}_0 - kx_0),$$

but we have an additional unknown, x_{-1}... if we write the finite differences approximation to $\dot{x}_0$ we can find an approximation to x_{-1} in terms of the initial velocity $\dot{x}_0$ and the unknown x_1

$$\dot{x}_0 = \frac{x_1 - x_{-1}}{2h} \Rightarrow x_{-1} = x_1 - 2h\dot{x}_0$$

Substituting in the previous equation and solving for x_1

$$x_1 = x_0 + h\dot{x}_0 + \frac{h^2}{2m}(p_0 - c\dot{x}_0 - kx_0)$$

Central differences

$$x_1 = x_0 + h\dot{x}_0 + \frac{h^2}{2m}(p_0 - c\dot{x}_0 - kx_0)$$

To start a new step, we need the value of $\dot{x}_1$, but we may approximate the mean velocity, again, by finite differences

$$\frac{\dot{x}_0 + \dot{x}_1}{2} = \frac{x_1 - x_0}{h} \Rightarrow \dot{x}_1 = \frac{2(x_1 - x_0)}{h} - \dot{x}_0$$

The method is very simple, but it is *conditionally stable*. The stability condition is defined with respect to the natural period of the SDOF oscillator,

$$h \leqslant \frac{T_n}{\pi} \approx 0.32 T_n$$

For a SDOF this is not relevant because, as we have seen in our previous example, we need more points for response cycle to correctly represent the response.

Methods based on Integration

We will make use of an *hypothesis* on the variation of the acceleration during the time step and of analytical integration of acceleration and velocity to step forward from the initial to the final condition for each time step.
In general, these methods are based on the two equations

$$\dot{x}_1 = \dot{x}_0 + \int_0^h \ddot{x}(\tau)\,d\tau,$$

$$x_1 = x_0 + \int_0^h \dot{x}(\tau)\,d\tau,$$

which express the final velocity and the final displacement in terms of the initial values x_0 and $\dot{x}_0$ and some definite integrals that depend on the *assumed* variation of the acceleration during the time step.

Integration Methods

Depending on the different assumption we can make on the variation of velocity, different integration methods can be derived.
We will see

- the constant acceleration method,
- the linear acceleration method,
- the family of methods known as *Newmark Beta Methods*, that comprises the previous methods as particular cases.

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Constant Acceleration

Here we assume that the acceleration is constant during each time step, equal to the mean value of the initial and final values:

$$\ddot{x}(\tau) = \ddot{x}_0 + \Delta\ddot{x}/2,$$

where $\Delta\ddot{x} = \ddot{x}_1 - \ddot{x}_0$, hence

$$\begin{aligned}
\dot{x}_1 &= \dot{x}_0 + \int_0^h (\ddot{x}_0 + \Delta\ddot{x}/2)\, d\tau \\
&\Rightarrow \Delta\dot{x} = \ddot{x}_0 h + \Delta\ddot{x} h/2 \\
x_1 &= x_0 + \int_0^h (\dot{x}_0 + (\ddot{x}_0 + \Delta\ddot{x}/2)\tau) d\tau \\
&\Rightarrow \Delta x = \dot{x}_0 h + (\ddot{x}_0) h^2/2 + \Delta\ddot{x} h^2/4
\end{aligned}$$

Constant acceleration

Taking into account the two equations on the right of the previous slide, and solving for $\Delta\dot{x}$ and $\Delta\ddot{x}$ in terms of Δx, we have

$$\Delta\dot{x} = \frac{2\Delta x - 2h\dot{x}_0}{h}, \quad \Delta\ddot{x} = \frac{4\Delta x - 4h\dot{x}_0 - 2\ddot{x}_0 h^2}{h^2}.$$

We have two equations and three unknowns... Assuming that the system characteristics are constant during a single step, we can write the equation of motion at times $\tau = h$ and $\tau = 0$, subtract member by member and write the *incremental equation of motion*

$$m\Delta\ddot{x} + c\Delta\dot{x} + k\Delta x = \Delta p,$$

that is a third equation that relates our unknowns.

Constant acceleration

Substituting the above expressions for $\Delta\dot{x}$ and $\Delta\ddot{x}$ in the incremental eq. of motion and solving for Δx gives, finally,

$$\Delta x = \frac{\tilde{p}}{\tilde{k}}, \qquad \Delta\dot{x} = \frac{2\Delta x - 2h\dot{x}_0}{h}$$

where

$$\tilde{k} = k + \frac{2c}{h} + \frac{4m}{h^2}$$

$$\tilde{p} = \Delta p + 2c\dot{x}_0 + m(2\ddot{x}_0 + \frac{4}{h}\dot{x}_0)$$

While it is possible to compute the final acceleration in terms of Δx, to achieve a better accuracy it is usually computed solving the equation of equilibrium written at the end of the time step.

Constant Acceleration

Two further remarks

The method is *unconditionally stable*

The effective stiffness, disregarding damping, is $\tilde{k} \approx k + 4m/h^2$.

If we consider that a) $m = k/\omega^2 = k/(2\pi/T)^2$ and b) we can relate h and T by the number n of time steps in a period, $h = T/n$, substituting these two relationships in the effective stiffness, we have

$$\tilde{k} \approx k(1 + \frac{n^2}{\pi^2})$$

For, e.g., a number of time steps in a period in the order of 6, the mass contribution to the effective stiffness is in the order of 80%.

Linear Acceleration

We assume that the acceleration is linear, i.e.

$$\ddot{x}(t) = \ddot{x}_0 + \Delta\ddot{x}\frac{\tau}{h}$$

hence

$$\Delta\dot{x} = \ddot{x}_0 h + \Delta\ddot{x}h/2, \quad \Delta x = \dot{x}_0 h + \ddot{x}_0 h^2/2 + \Delta\ddot{x}h^2/6$$

Following a derivation similar to what we have seen in the case of constant acceleration, we can write, again,

$$\Delta x = \left(k + 3\frac{c}{h} + 6\frac{m}{h^2}\right)^{-1}\left[\Delta p + c(\ddot{x}_0\frac{h}{2} + 3\dot{x}_0) + m(3\ddot{x}_0 + 6\frac{\dot{x}_0}{h})\right]$$

$$\Delta\dot{x} = \Delta x\frac{3}{h} - 3\dot{x}_0 - \ddot{x}_0\frac{h}{2}$$

Linear Acceleration

The linear acceleration method is *conditionally stable*, the stability condition being

$$\frac{h}{T} \leqslant \frac{\sqrt{3}}{\pi} \approx 0.55$$

When dealing with SDOF systems, this condition is never of concern, as we need a shorter step to accurately describe the response of the oscillator, let's say $h \leqslant 0.12T$...

When stability is not a concern, the accuracy of the linear acceleration method is far superior to the accuracy of the constant acceleration method, so that this is the method of choice for the analysis of SDOF systems.

Newmark Beta Methods

The constant and linear acceleration methods are just two members of the family of Newmark Beta methods, where we write

$$\Delta\dot{x} = (1-\gamma)h\ddot{x}_0 + \gamma h\ddot{x}_1$$

$$\Delta x = h\dot{x}_0 + (\frac{1}{2} - \beta)h^2\ddot{x}_0 + \beta h^2\ddot{x}_1$$

The factor γ weights the influence of the initial and final accelerations on the velocity increment, while β has a similar role with respect to the displacement increment.

Newmark Beta Methods

Using $\gamma \neq 1/2$ leads to numerical damping, so when analysing SDOF systems, one uses $\gamma = 1/2$ (numerical damping may be desirable when dealing with MDOF systems).
Using $\beta = \frac{1}{4}$ leads to the constant acceleration method, while $\beta = \frac{1}{6}$ leads to the linear acceleration method. In the context of MDOF analysis, it's worth knowing what is the minimum β that leads to an unconditionally stable behaviour.

Newmark Beta Methods

The general format for the solution of the incremental equation of motion using the Newmark Beta Method can be written as follows:

$$\Delta x = \frac{\Delta \tilde{p}}{\tilde{k}}$$

$$\Delta v = \frac{\gamma}{\beta}\frac{\Delta x}{h} - \frac{\gamma}{\beta}v_0 + h\left(1 - \frac{\gamma}{2\beta}\right)a_0$$

with

$$\tilde{k} = k + \frac{\gamma}{\beta}\frac{c}{h} + \frac{1}{\beta}\frac{m}{h^2}$$

$$\Delta \tilde{p} = \Delta p + \left(h\left(\frac{\gamma}{2\beta} - 1\right)c + \frac{1}{2\beta}m\right)a_0 + \left(\frac{\gamma}{\beta}c + \frac{1}{\beta}\frac{m}{h}\right)v_0$$

Non Linear Systems

A convenient procedure for integrating the response of a non linear system is based on the incremental formulation of the equation of motion, where for the stiffness and the damping were taken values representative of their variation during the time step: in line of principle, the mean values of stiffness and damping during the time step, or, as this is usually not possible, their initial values, k_0 and c_0.
The Newton-Raphson method can be used to reduce the unbalanced forces at the end of the step.

Non Linear Systems

Usually we use the modified Newton-Raphson method, characterised by not updating the system stiffness at each iteration. In pseudo-code, referring for example to the Newmark Beta Method

```
x1,v1,f1 = x0,v0,f0 % initialisation; gb=gamma/beta
Dr = DpTilde
loop:
   Dx = Dr/kTilde
   x2 = x1 + Dx
   v2 = gb*Dx/h + (1-gb)*v1 + (1-gb/2)*h*a0
   x_pl = update_u_pl(...)
   f2 = k*(x2-x_pl)
   % important
   Df = (f2-f1) + (kTilde-k_ini)*Dx
   Dr = Dr - Df
   x1, v1, f1 = x2, v2, f2
   if ( tol(...) < req_tol ) BREAK loop
```

Exercise

A system has a mass $\mathrm{m} = 1000\mathrm{kg}$, a stiffness $\mathrm{k} = 40000\mathrm{N/m}$ and a viscous damping whose ratio to the critical damping is $\zeta = 0.03$.
The spring is elastoplastic, with a yielding force of 2500N.
The load is an half-sine impulse, with duration 0.3s and maximum value of 6000N.
Use the constant acceleration method to integrate the response, with $\mathrm{h} = 0.05\mathrm{s}$ and, successively, $\mathrm{h} = 0.02\mathrm{s}$. Note that the stiffness is either 0 or k, write down the expression for the effective stiffness and loading in the incremental formulation, write a spreadsheet or a program to make the computations.