Multi Degrees of Freedom Systems MDOF's

Giacomo Boffi

Dipartimento di Ingegneria Strutturale, Politecnico di Milano

April 29, 2010

Generalized SDOF's

Giacomo Boffi

Introductory Remarks

The Homogeneous Problem

Modal Analysis

Examples

The Homogeneous Problem

Modal Analysis

Examples

Introductory Remarks

An Example

The Equation of Motion, a System of Linear Differential Equations

Matrices are Linear Operators

Properties of Structural Matrices

An example

The Homogeneous Problem

The Homogeneous Equation of Motion

Eigenvalues and Eigenvectors

Eigenvectors are Orthogonal

Modal Analysis

Eigenvectors are a base

EoM in Modal Coordinates

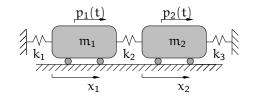
Initial Conditions

Examples

2 DOF System



Consider an undamped system with two masses and two degrees of freedom,



write the equation of equilibrium, using the D'Alembert principle, for each mass:

Introductory Remarks

An Example
The Equation of
Motion
Matrices are
Linear Operators

Linear Operat
Properties of
Structural
Matrices
An example

The Homogeneous Problem

Modal Analysis

Examples

With some little rearrangement we have a system of two linear differential equations in two variables, $x_1(t)$ and $x_2(t)$:

$$\begin{cases} m_1\ddot{x}_1 + (k_1 + k_2)x_1 - k_3x_2 &= p_1(t) \\ m_1\ddot{x}_1 - k_2x_1 + (k_2 + k_3)x_2 &= p_2(t). \end{cases}$$

An Example
The Equation of
Motion
Matrices are

Linear Operators Properties of Structural Matrices

An example

The Homogeneous Problem

Modal Analysis

Examples

Introducing the loading vector \mathbf{p} , the vector of inertial forces \mathbf{f}_{I} and the vector of elastic forces \mathbf{f}_{S} ,

$$p = \left\{ \begin{matrix} p_1(t) \\ p_2(t) \end{matrix} \right\}, \quad f_I = \left\{ \begin{matrix} f_{I,1} \\ f_{I,2} \end{matrix} \right\}, \quad f_S = \left\{ \begin{matrix} f_{S,1} \\ f_{S,2} \end{matrix} \right\}$$

we can write a vectorial equation of equilibrium:

$$f_I + f_S = p(t).$$

It is possible to write the linear relationship between $f_{\rm S}$ and the vector of displacements

$$\mathbf{x} = egin{cases} \mathbf{x}_1 \\ \mathbf{x}_2 \end{bmatrix}$$
 ,

in terms of a matrix product

$$\mathbf{f}_S = \begin{bmatrix} k_1 + k_2 & -k_2 \\ -k_2 & k_2 + k_3 \end{bmatrix} \mathbf{x}$$

or, introducing the stiffness matrix K,

$$\mathbf{K} = \begin{bmatrix} k-1+k_2 & -k_2 \\ -k_2 & k_2+k+3 \end{bmatrix},$$

we can write

$$f_S = K x$$

Introductory Remarks

An Example
The Equation of
Motion
Matrices are
Linear Operators

Properties of Structural Matrices

The Homogeneous Problem

Modal Analysis

Examples

Introductory Remarks

An Example
The Equation of
Motion
Matrices are
Linear Operators

Properties of Structural Matrices An example

The Homogeneous

Modal Analysis

Examples

Problem

Analogously, introducing the mass matrix M

$$\mathbf{M} = \begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix}$$

we can write

$$f_I = M \ddot{x}$$
.

matricial format:

Finally it is possible to write the equation of motion in

$$M\ddot{x} + Kx = p(t).$$

In the following we will see how it is possible to consider the effects o damping introducing a *damping matrix* C and writing

$$M\ddot{x} + C\dot{x} + Kx = p(t),$$

however it is now more productive fixing our attention on undamped systems.

Introductory Remarks

An Example The Equation of

The Equation of Motion

Matrices are
Linear Operators

Properties of Structural Matrices

An example
The
Homogeneous

Problem

Modal Analysis

vioual Allalysis

Examples

Introductory Remarks

An Example
The Equation of
Motion
Matrices are
Linear Operators

Properties of Structural Matrices An example

The Homogeneous Problem

Modal Analysis

Examples

Finally it is possible to write the equation of motion in matricial format:

$$M\ddot{x} + Kx = p(t).$$

In the following we will see how it is possible to consider the effects of damping introducing a $damping\ matrix\ C$ and writing

$$M\ddot{x} + C\dot{x} + Kx = p(t),$$

however it is now more productive fixing our attention on undamped systems.

An Example
The Equation of
Motion
Matrices are
Linear Operators
Properties of
Structural

Matrices An example

The Homogeneous Problem

Modal Analysis

Examples

▶ if **K** were symmetrical, the force on mass j due to an unit displacement of mass i would be equal to the force on mass i due to an unit displacement of mass j; as this is true because the two masses are joined by the same spring, we have that **K** is symmetrical.

▶ The strain energy V for a discrete system can be written

$$V = \frac{1}{2} \mathbf{x}^\mathsf{T} \mathbf{f}_\mathsf{S} = \frac{1}{2} \mathbf{x}^\mathsf{T} \mathbf{K} \mathbf{x},$$

because the strain energy is positive it follows that K is a positive definite matrix.

▶ if **K** were symmetrical, the force on mass j due to an unit displacement of mass i would be equal to the force on mass i due to an unit displacement of mass j; as this is true because the two masses are joined by the same spring, we have that **K** is symmetrical.

► The strain energy V for a discrete system can be written

$$V = \frac{1}{2} \mathbf{x}^\mathsf{T} \mathbf{f}_S = \frac{1}{2} \mathbf{x}^\mathsf{T} \mathbf{K} \mathbf{x},$$

because the strain energy is positive it follows that ${\bf K}$ is a positive definite matrix.

Restricting our discussion to systems whose degrees of freedom are the displacements of a set of discrete masses, we have that the mass matrix is a diagonal matrix, with all its diagonal elements greater than zero. Such a matrix is symmetrical and definite positive, as well as the stiffness matrix is symmetrical and definite positive.

En passant, take note that the kinetic energy for a discrete system is

$$T = \frac{1}{2} \dot{\mathbf{x}}^{\mathsf{T}} \mathbf{M} \, \dot{\mathbf{x}}.$$

system is

An Example
The Equation of
Motion
Matrices are
Linear Operators
Properties of
Structural

Matrices An example

The Homogeneous Problem

Modal Analysis

Examples

Restricting our discussion to systems whose degrees of freedom are the displacements of a set of discrete masses, we have that the mass matrix is a diagonal matrix, with all its diagonal elements greater than zero. Such a matrix is symmetrical and definite positive, as well as the stiffness matrix is symmetrical and definite positive. En passant, take note that the kinetic energy for a discrete

$$T = \frac{1}{2} \dot{\mathbf{x}}^{\mathsf{T}} \mathbf{M} \, \dot{\mathbf{x}}.$$

Introductory Remarks An Example

An Example
The Equation of
Motion
Matrices are
Linear Operators
Properties of

Structural Matrices

An example

The Homogeneous Problem

Modal Analysis

Examples

The findings in the previous two slides can be generalised to the *structural matrices* of generic structural systems, with one exception.

For a general structural system, M could be semi-definite positive, that is for some particular displacement vector the kinetic energy could be zero.

Giacomo Boffi

Introductory Remarks

An Example
The Equation of
Motion
Matrices are
Linear Operators
Properties of

Structural Matrices

An example

The Homogeneous Problem

Modal Analysis

Examples

The findings in the previous two slides can be generalised to the *structural matrices* of generic structural systems, with one exception.

For a general structural system, M could be *semi-definite* positive, that is for some particular displacement vector the kinetic energy could be zero.

Giacomo Boffi



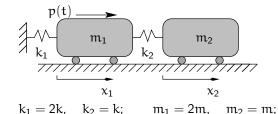
An Example
The Equation of
Motion
Matrices are
Linear Operators
Properties of
Structural
Matrices

An example

The Homogeneous Problem

Modal Analysis

Examples



 $p(t) = p_0 \sin \omega t$.

The solution

Generalized SDOF's

Giacomo Boffi

Introductory Remarks

An Example
The Equation of
Motion
Matrices are
Linear Operators
Properties of
Structural
Matrices

An example

The Homogeneous Problem

Modal Analysis

Examples

. . .

Giacomo Boffi



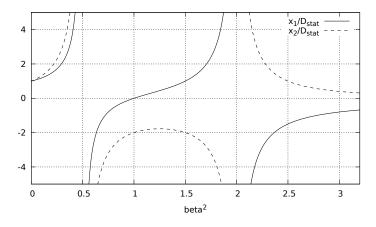
An Example
The Equation of
Motion
Matrices are
Linear Operators
Properties of
Structural
Matrices

An example

The Homogeneous Problem

Modal Analysis

Examples



The

Orthogonal

To understand the behaviour of a *MDOF* system, we start writing the homogeneous equation of motion,

$$\mathbf{M}\ddot{\mathbf{x}} + \mathbf{K}\mathbf{x} = \mathbf{0},$$

and use the technique of separation of variables

$$x(t) = \psi(A\sin\omega t + B\cos\omega t)$$

where ψ is a fixed, unknown vector, named a *shape vector*.

Substituting in the equation of motion, we have

$$(\mathbf{K} - \omega^2 \mathbf{M}) \, \mathbf{\psi}(\mathbf{A} \sin \omega \mathbf{t} + \mathbf{B} \cos \omega \mathbf{t}) = 0$$

The

To understand the behaviour of a *MDOF* system, we start writing the homogeneous equation of motion,

$$\mathbf{M}\ddot{\mathbf{x}} + \mathbf{K}\mathbf{x} = \mathbf{0},$$

and use the technique of separation of variables

$$x(t) = \psi(A\sin\omega t + B\cos\omega t)$$

where ψ is a fixed, unknown vector, named a *shape vector*. Substituting in the equation of motion, we have

$$(\mathbf{K} - \omega^2 \mathbf{M}) \, \mathbf{\psi}(\mathbf{A} \sin \omega t + \mathbf{B} \cos \omega t) = 0$$

Eigenvalues

Generalized SDOF's

Introductory Remarks

The Homogeneous Problem Homogeneous

Equation of Motion

Orthogonal Modal Analysis

Examples

Giacomo Boffi

The

Eigenvalues and Eigenvectors Eigenvectors are

The previous equation must hold for every value of t, so it can be reduced to

$$\left(K - \omega^2 M\right)\psi = 0$$

$$\psi = 0$$

$$\det\left(\mathbf{K} - \omega^2 \mathbf{M}\right) = 0$$

Eigenvalues

Generalized SDOF's

Giacomo Boffi

Introductory Remarks

The Homogeneous Problem

Homogeneous
Equation of
Motion
Eigenvalues and
Eigenvectors

Eigenvectors are Orthogonal Modal Analysis

. .

Examples

The previous equation must hold for every value of t, so it can be reduced to

$$\left(K - \omega^2 M\right)\psi = 0$$

We have a homogeneous linear equation, with unknowns ψ_i and the matrix of coefficients that depends on the parameter ω^2 .

The trivial solution being

$$\psi = 0$$
,

different solutions are available when

$$\det\left(\mathbf{K} - \omega^2 \mathbf{M}\right) = 0$$

The *eigenvalues* of the *MDOF* system are the values of ω^2 for which the above equation is verified.

Eigenvalues

Generalized SDOF's

Giacomo Boffi

Problem The

Introductory Remarks The Homogeneous

Homogeneous Equation of Motion Eigenvalues and

Eigenvectors Eigenvectors are Orthogonal

The previous equation must hold for every value of t, so it can be reduced to

$$\left(K - \omega^2 M\right)\psi = 0$$

We have a homogeneous linear equation, with unknowns ψ_i and the matrix of coefficients that depends on the parameter ω^2 .

The trivial solution being

$$\psi = 0$$
,

different solutions are available when

$$\det\left(\mathbf{K} - \omega^2 \mathbf{M}\right) = 0$$

The eigenvalues of the MDOF system are the values of ω^2 for which the above equation is verified.

The

Homogeneous Equation of Motion Eigenvalues and Eigenvectors

Eigenvectors are Orthogonal Modal Analysis

Examples

For a system with N degrees of freedom the expansion of det $(\mathbf{K} - \omega^2 \mathbf{M})$ is an algebraic polynomial of degree N in ω^2 , whose roots, ω_i^2 , $i=1,\ldots,N$ are all real and greater than zero.

Substituting one of the roots $\omega_{\hat{\iota}}^2$ in the characteristic equation,

$$\left(K-\omega_{\mathfrak{i}}^{2}M\right)\psi_{\mathfrak{i}}=0$$

each one of the N eigenvectors ψ_i can be computed, except for an undetermined common scale factor.

A common choice for the normalisation of the eigenvectors is normalisation with respect to the mass matrix,

$$\psi_i^T M \psi_i = 1$$

The

Homogeneous
Equation of
Motion
Eigenvalues and
Eigenvectors

Eigenvectors are Orthogonal

Modal Analysis

Examples

The most general expression (general integral) for the displacement of an homogeneous system is

$$x(t) = \sum_{i=1}^N \psi_i(A_i \sin \omega_i t + B_i \cos \omega_i t)$$

In the general integral there are 2N unknown *constants of integration*, that must be determined in terms of the initial conditions, usually expressed in terms of initial displacements and initial velocities,

$$\begin{cases} x(0) = x_0 \\ \dot{x}(0) = \dot{x}_0 \end{cases} \quad \Rightarrow \quad \begin{cases} x_{\mathfrak{i},0} = \sum_{\mathfrak{j}=1}^{N} \ \psi_{\mathfrak{i}\mathfrak{j}} B_{\mathfrak{j}} \\ \dot{x}_{\mathfrak{i},0} = \sum_{\mathfrak{j}=1}^{N} \omega_{\mathfrak{j}} \psi_{\mathfrak{i}\mathfrak{j}} A_{\mathfrak{j}} \end{cases} \quad \text{for } \mathfrak{i} = 1, \ldots, N,$$

where ψ_{ij} is the i-nth component of ψ_j .

Giacomo Boffi

Introductory Remarks

The Homogeneous Problem

The Homogeneous Equation of Motion

Eigenvalues and Eigenvectors Eigenvectors are

Orthogonal Modal Analysis

Examples

i forgot to link the pdf with the last week lesson... i will put the link in place tonight, but if you don't trust me, after the class come here with your USB key, you'll be welcome!

Introductory Remarks

The Homogeneous Problem

The
Homogeneous
Equation of
Motion
Eigenvalues and
Eigenvectors
Eigenvectors are
Orthogonal

Modal Analysis

Take into consideration two distinct eigenvalues, ω_r^2 and ω_s^2 , and write the characteristic equation for each eigenvalue:

$$\begin{split} K \psi_r &= \omega_r^2 M \, \psi_r \\ K \psi_s &= \omega_s^2 M \, \psi_s \end{split}$$

premultiply each equation member by the transpose of the *other* eigenvector

$$\psi_s^\mathsf{T} \mathbf{K} \psi_r = \omega_r^2 \psi_s^\mathsf{T} \mathbf{M} \psi_r$$
$$\psi_r^\mathsf{T} \mathbf{K} \psi_s = \omega_s^2 \psi_r^\mathsf{T} \mathbf{M} \psi_s$$

Take into consideration two distinct eigenvalues, ω_r^2 and ω_s^2 , and write the characteristic equation for each eigenvalue:

$$K\psi_r = \omega_r^2 M \psi_r$$
$$K\psi_s = \omega_s^2 M \psi_s$$

premultiply each equation member by the transpose of the *other* eigenvector

$$\begin{split} \boldsymbol{\psi}_s^\mathsf{T} \boldsymbol{K} \boldsymbol{\psi}_r &= \boldsymbol{\omega}_r^2 \boldsymbol{\psi}_s^\mathsf{T} \boldsymbol{M} \, \boldsymbol{\psi}_r \\ \boldsymbol{\psi}_r^\mathsf{T} \boldsymbol{K} \boldsymbol{\psi}_s &= \boldsymbol{\omega}_s^2 \boldsymbol{\psi}_r^\mathsf{T} \boldsymbol{M} \, \boldsymbol{\psi}_s \end{split}$$

Introductory Remarks

The Homogeneous Problem

The Homogeneous Equation of Motion Eigenvalues and Eigenvectors Eigenvectors are Orthogonal

Modal Analysis

Examples

The term $\psi_s^\mathsf{T} K \psi_r$ is a scalar, hence

$$\boldsymbol{\psi}_s^\mathsf{T} \boldsymbol{K} \boldsymbol{\psi}_r = \left(\boldsymbol{\psi}_s^\mathsf{T} \boldsymbol{K} \boldsymbol{\psi}_r\right)^\mathsf{T} = \boldsymbol{\psi}_r^\mathsf{T} \boldsymbol{K}^\mathsf{T} \boldsymbol{\psi}_s$$

but K is symmetrical, $K^{\mathsf{T}} = K$ and we have

$$\psi_s^\mathsf{T} K \psi_r = \psi_r^\mathsf{T} K \psi_s.$$

By a similar derivation

$$\psi_s^T M \, \psi_r = \psi_r^T M \, \psi_s.$$

The

Substituting our last identities in the previous equations, we have

$$\psi_r^\mathsf{T} \mathbf{K} \psi_s = \omega_r^2 \psi_r^\mathsf{T} \mathbf{M} \psi_s$$
$$\psi_r^\mathsf{T} \mathbf{K} \psi_s = \omega_s^2 \psi_r^\mathsf{T} \mathbf{M} \psi_s$$

subtracting member by member we find that

$$(\omega_r^2 - \omega_s^2) \, \psi_r^T M \, \psi_s = 0$$

$$\psi_r^T M \psi_s = 0, \quad \text{for } r \neq s.$$

The

Substituting our last identities in the previous equations, we have

$$\psi_r^\mathsf{T} \mathbf{K} \psi_s = \omega_r^2 \psi_r^\mathsf{T} \mathbf{M} \psi_s$$
$$\psi_r^\mathsf{T} \mathbf{K} \psi_s = \omega_s^2 \psi_r^\mathsf{T} \mathbf{M} \psi_s$$

subtracting member by member we find that

$$(\omega_r^2 - \omega_s^2) \, \psi_r^T M \, \psi_s = 0$$

We started with the hypothesis that $\omega_r^2 \neq \omega_s^2$, so for every $r \neq s$ we have that the corresponding eigenvectors are orthogonal with respect to the mass matrix

$$\psi_r^\mathsf{T} M \, \psi_s = 0, \qquad \text{for } r \neq s.$$

The Homogeneous Equation of Motion Eigenvalues and Eigenvectors Eigenvectors are Orthogonal

Modal Analysis

Examples

The eigenvectors are orthogonal also with respect to the stiffness matrix:

$$\label{eq:psi_spin_substitute} \boldsymbol{\psi}_s^\mathsf{T} \boldsymbol{K} \boldsymbol{\psi}_r = \boldsymbol{\omega}_r^2 \boldsymbol{\psi}_s^\mathsf{T} \boldsymbol{M} \, \boldsymbol{\psi}_r = \textbf{0}, \quad \text{for } r \neq s.$$

By definition

$$M_i = \psi_i^T M \psi_i$$

anc

$$\psi_i^\mathsf{T} \mathbf{K} \psi_i = \omega_i^2 M_i$$

The Homogeneous Equation of Motion Eigenvalues and Eigenvectors Eigenvectors are Orthogonal

Modal Analysis

SDOF S

The eigenvectors are orthogonal also with respect to the stiffness matrix:

$$\label{eq:psi_spin_substitute} \boldsymbol{\psi}_s^\mathsf{T} \boldsymbol{K} \boldsymbol{\psi}_r = \boldsymbol{\omega}_r^2 \boldsymbol{\psi}_s^\mathsf{T} \boldsymbol{M} \, \boldsymbol{\psi}_r = \textbf{0}, \quad \text{for } r \neq s.$$

By definition

$$M_{i} = \psi_{i}^{\mathsf{T}} M \psi_{i}$$

and

$$\psi_i^T K \psi_i = \omega_i^2 M_i.$$

The eigenvector are linearly independent, so for every vector x we can write

$$x = \sum_{j=1}^N \psi_j q_j, \quad \text{with } q_j = \frac{\psi_j^\mathsf{T} M \, x}{M_j}$$

because of orthogonality and, generalising,

$$\begin{split} \boldsymbol{x}(t) &= \sum_{j=1}^N \psi_j q_j(t), & \ddot{\boldsymbol{x}}(t) &= \sum_{j=1}^N \psi_j \ddot{q}_j(t), \\ \boldsymbol{x}_i(t) &= \sum_{j=1}^N \Psi_{ij} q_j(t), \\ \boldsymbol{x}(t) &= \boldsymbol{\Psi} \, \boldsymbol{q}(t), & \ddot{\boldsymbol{x}}(t) &= \boldsymbol{\Psi} \, \ddot{\boldsymbol{q}}(t). \end{split}$$

where q(t) is the vector of modal coordinates and Ψ , whose columns are the eigenvectors, is the eigenvector matrix.

base

EoM in Modal

Substitute in the equation of motion,

$$M\,\Psi\,\ddot{q}+K\,\Psi\,q=p(t)$$

premultiply by Ψ^{I}

$$\boldsymbol{\Psi}^{\mathsf{T}} \boldsymbol{M} \, \boldsymbol{\Psi} \, \ddot{\boldsymbol{q}} + \boldsymbol{\Psi}^{\mathsf{T}} \boldsymbol{K} \, \boldsymbol{\Psi} \, \boldsymbol{q} = \boldsymbol{\Psi}^{\mathsf{T}} \boldsymbol{p}(t)$$

with obvious definitions

$$M^\star \ddot{q} + K^\star \, q = p^\star(t)$$

Modal Analysis Eigenvectors are a base EoM in Modal

Initial Conditions

Examples

By the preceding obvious definitions we have that the generic element of the starred matrices can be expressed in terms of single eigenvectors,

$$\begin{split} M_{ij}^{\star} &= \boldsymbol{\psi}_{i}^{\mathsf{T}} \boldsymbol{M} \, \boldsymbol{\psi}_{j} = \boldsymbol{\delta}_{ij} M_{i}, \\ K_{ii}^{\star} &= \boldsymbol{\psi}_{i}^{\mathsf{T}} \boldsymbol{K} \, \boldsymbol{\psi}_{i} = \boldsymbol{\omega}_{i}^{2} \boldsymbol{\delta}_{ij} M_{i}. \end{split}$$

where δ_{ij} is the *Kroneker* symbol,

$$\delta_{ij} = \begin{cases} 1 & i = j \\ 0 & i \neq j \end{cases}$$

$$M_i \ddot{q}_i + \omega_i^2 M_i q_i = p_i^\star(t), \qquad i = 1, \dots, N$$

By the preceding obvious definitions we have that the generic element of the starred matrices can be expressed in terms of single eigenvectors,

$$M_{ij}^{\star} = \psi_{i}^{\mathsf{T}} M \psi_{j} = \delta_{ij} M_{i},$$

$$K_{ij}^{\star} = \psi_{i}^{\mathsf{T}} K \psi_{j} = \omega_{i}^{2} \delta_{ij} M_{i}.$$

where δ_{ij} is the *Kroneker* symbol,

$$\delta_{ij} = \begin{cases} 1 & i = j \\ 0 & i \neq j \end{cases}$$

Substituting in the equation of motion, with $\mathbf{p}_{i}^{\star} = \mathbf{\psi}_{i}^{\mathsf{T}} \mathbf{p}(t)$ we have a set of uncoupled equations

$$M_i\ddot{q}_i + \omega_i^2 M_i q_i = p_i^*(t), \qquad i = 1, ..., N$$

Introductory Remarks

Homogeneous Problem Modal Analysis

The initial conditions

$$\begin{cases} x(0) = x_0 \\ \dot{x}(0) = \dot{x}_0 \end{cases}$$

Consider, e.g., the initial displacements: we can write

$$x_0 = \Psi q_0$$

premultiplying both members by $\Psi^{T}M$,

$$\boldsymbol{\Psi}^{\mathsf{T}} \boldsymbol{M} \, \boldsymbol{x}_0 = \boldsymbol{\Psi}^{\mathsf{T}} \boldsymbol{M} \, \boldsymbol{\Psi} \, \boldsymbol{q}_0 = \boldsymbol{M}^{\star} \boldsymbol{q}_0$$

premultiplying by the inverse of \mathbf{M}^{\star} and taking into account that M^* is diagonal,

$$q_0 = (M^\star)^{-1} \Psi^\mathsf{T} M \, x_0 \quad \Rightarrow \quad q_{i0} = \frac{\psi_i^\mathsf{T} M \, x_0}{M_i}$$

analogously

$$\dot{q}_{i0} = \frac{{\psi_i}^T M \, \dot{x}_0}{M_{i_{\text{const}}}}$$

Giacomo Boffi



The Homogeneous Problem

Modal Analysis

Examples
2 DOF System

$$k_1=k,\quad k_2=2k;\qquad m_1=2m,\quad m_2=m;$$

$$p(t)=p_0\sin\omega t.$$

$$\begin{split} \mathbf{x} &= \left\{ \begin{matrix} x_1 \\ x_2 \end{matrix} \right\}, \ \mathbf{p}(t) = \left\{ \begin{matrix} 0 \\ p_0 \end{matrix} \right\} \sin \omega t, \\ \mathbf{M} &= \mathbf{m} \begin{bmatrix} 2 & 0 \\ 0 & 1 \end{bmatrix}, \ \mathbf{K} = \mathbf{k} \begin{bmatrix} 3 & -2 \\ -2 & 2 \end{bmatrix}. \end{split}$$

The

Modal Analysis

Examples 2 DOF System

Characteristic Equation

The characteristic equation is

$$\left\| \boldsymbol{K} - \omega^2 \boldsymbol{M} \right\| = \left\| \begin{matrix} 3k - 2\omega^2 m & -2k \\ -2k & 2k - \omega^2 m \end{matrix} \right\| = 0.$$

$$(2m^2)\omega^4 - (7mk)\omega^2 + (2k^2)\omega^0 = 0$$

$$\omega_1^2 = \frac{k}{m} \frac{7 - \sqrt{33}}{4} \qquad \qquad \omega_2^2 = \frac{k}{m} \frac{7 + \sqrt{33}}{4}$$

$$\omega_1^2 = 0.31386 \frac{k}{m} \qquad \qquad \omega_2^2 = 3.18614 \frac{k}{m}$$

The

Homogeneous Problem

Modal Analysis

Examples 2 DOF System

The characteristic equation is

$$\left\|\boldsymbol{K} - \omega^2 \boldsymbol{M} \right\| = \left\| \begin{matrix} 3k - 2\omega^2 m & -2k \\ -2k & 2k - \omega^2 m \end{matrix} \right\| = 0.$$

Developing the determinant

$$(2m^2)\omega^4 - (7mk)\omega^2 + (2k^2)\omega^0 = 0$$

$$\omega_1^2 = \frac{k}{m} \frac{7 - \sqrt{33}}{4} \qquad \qquad \omega_2^2 = \frac{k}{m} \frac{7 + \sqrt{33}}{4}$$

$$\omega_1^2 = 0.31386 \frac{k}{m} \qquad \qquad \omega_2^2 = 3.18614 \frac{k}{m}$$

The

Homogeneous Problem

Modal Analysis

Examples 2 DOF System

The characteristic equation is

$$\left\|\boldsymbol{K} - \omega^2 \boldsymbol{M} \right\| = \left\| \begin{matrix} 3k - 2\omega^2 m & -2k \\ -2k & 2k - \omega^2 m \end{matrix} \right\| = 0.$$

Developing the determinant

$$(2m^2)\omega^4 - (7mk)\omega^2 + (2k^2)\omega^0 = 0$$

Solving the algebraic equation in ω^2

$$\omega_1^2 = \frac{k}{m} \frac{7 - \sqrt{33}}{4} \qquad \qquad \omega_2^2 = \frac{k}{m} \frac{7 + \sqrt{33}}{4}$$

$$\omega_1^2 = 0.31386 \frac{k}{m} \qquad \qquad \omega_2^2 = 3.18614 \frac{k}{m}$$

The first of the characteristic equation, substituting ω_1^2 , gives

$$k\,(3-2\times 0.31386)\psi_{11}-2k\psi_{21}=0$$

while substituting ω_2^2 gives

$$k(3-2\times 3.18614)\psi_{12}-2k\psi_{22}=0$$

solving with $\psi_{21} = \psi_{22} = 1$ gives

$$\psi_1 = \begin{cases} +0.84307 \\ +1.00000 \end{cases}, \quad \psi_2 = \begin{cases} -0.59307 \\ +1.00000 \end{cases},$$

the unnormalized eigenvectors.

The Homogeneous Problem

Modal Analysis

Examples 2 DOF System

We compute first M_1 and M_2 ,

$$\begin{split} M_1 &= \psi_1^T M \, \psi_1 \\ &= \left\{0.84307, \quad 1\right\} \begin{bmatrix} 2m & 0 \\ 0 & m \end{bmatrix} \left\{ \begin{matrix} 0.84307 \\ 1 \end{matrix} \right\} \\ &= \left\{1.68614m, \quad m\right\} \left\{ \begin{matrix} 0.84307 \\ 1 \end{matrix} \right\} = 2.42153m \end{split}$$

$$M_2 = 1.70346 m$$

the adimensional normalisation factors are

$$\alpha_1 = \sqrt{2.42153}, \qquad \alpha_2 = \sqrt{1.70346}.$$

Applying the normalisation factors to the respective unnormalised eigenvectors and collecting them in a matrix, we have the matrix of normalized eigenvectors

$$\Psi = \begin{bmatrix} +0.54177 & -0.45440 \\ +0.64262 & +0.76618 \end{bmatrix}$$

The Homogeneous Problem

Modal Analysis

Examples
2 DOF System

The modal loading is

$$\begin{split} \boldsymbol{p}^{\star}(t) &= \boldsymbol{\Psi}^{T} \; \boldsymbol{p}(t) \\ &= p_{0} \; \begin{bmatrix} +0.54177 & +0.64262 \\ -0.45440 & +0.76618 \end{bmatrix} \; \begin{cases} 0 \\ 1 \end{cases} \sin \omega t \\ &= p_{0} \; \begin{cases} +0.64262 \\ +0.76618 \end{cases} \; \sin \omega t \end{split}$$

The Homogeneous Problem

Modal Analysis

Examples
2 DOF System

Substituting its modal expansion for x into the equation of motion and premultiplying by Ψ^T we have the uncoupled modal equation of motion

$$\begin{cases} m\ddot{q}_1 + 0.31386k\,q_1 = +0.64262\,p_0\sin\omega t \\ m\ddot{q}_2 + 3.18614k\,q_2 = +0.76618\,p_0\sin\omega t \end{cases}$$

Note that all the terms are dimensionally correct. Dividing by $\mathfrak m$ both equations, we have

$$\begin{cases} \ddot{q}_1 + \omega_1^2 q_1 = +0.64262 \frac{p_0}{m} \sin \omega t \\ \ddot{q}_2 + \omega_2^2 q_2 = +0.76618 \frac{p_0}{m} \sin \omega t \end{cases}$$

$$\xi_1 = C_1 \sin \omega t, \quad \ddot{\xi} = -\omega^2 C_1 \sin \omega t$$

and substitute in the first modal EoM:

$$C_1 \left(\omega_1^2 - \omega^2 \right) \sin \omega t = \frac{p_1^\star}{m} \sin \omega t$$

solving for C_1

$$C_1 = \frac{p_1^\star}{m} \frac{1}{\omega_1^2 - \omega^2}$$

with $\omega_1^2 = K_1/m \ \Rightarrow \ m = K_1/\omega_1^2$:

$$C_1 = \frac{p_1^\star}{K_1} \frac{\omega_1^2}{\omega_1^2 - \omega^2} = \Delta_{\text{st}}^{(1)} \frac{1}{1 - \beta_1^2} \quad \text{with } \Delta_{\text{st}}^{(1)} = \frac{p_1^\star}{K_1} = 2.047 \frac{p_0}{k} \text{ and } \beta_1 = \frac{\omega}{\omega_1}$$

of course

$$C_2 = \Delta_{\text{st}}^{(2)} \frac{1}{1-\beta_2^2} \quad \text{with } \Delta_{\text{st}}^{(2)} = \frac{p_2^\star}{K_2} = 0.2404 \frac{p_0}{k} \text{ and } \beta_2 = \frac{\omega}{\omega_2}$$

The Homogeneous Problem

Modal Analysis

Examples
2 DOF System

4□ > 4□ > 4 = > 4 = > = 9 < 0</p>

$$\left\{ \begin{aligned} q_1(t) &= A_1 \sin \omega_1 t + B_1 \cos \omega_1 t + \Delta_{\text{st}}^{(1)} \frac{\sin \omega t}{1-\beta_1^2} \\ q_2(t) &= A_2 \sin \omega_2 t + B_2 \cos \omega_2 t + \Delta_{\text{st}}^{(2)} \frac{\sin \omega t}{1-\beta_2^2} \end{aligned} \right.$$

for a system initially at rest

$$\left\{ \begin{aligned} q_1(t) &= \Delta_{\text{st}}^{(1)} \frac{1}{1-\beta_1^2} \left(\sin \omega t - \beta_1 \sin \omega_1 t \right) \\ q_2(t) &= \Delta_{\text{st}}^{(2)} \frac{1}{1-\beta_2^2} \left(\sin \omega t - \beta_2 \sin \omega_2 t \right) \end{aligned} \right.$$

we are interested in structural degrees of freedom, too... disregarding transient

$$\begin{cases} x_1(t) = \left(\psi_{11} \frac{\Delta_{\text{st}}^{(1)}}{1 - \beta_1^2} + \psi_{12} \frac{\Delta_{\text{st}}^{(2)}}{1 - \beta_2^2} \right) \sin \omega t = \left(\frac{1.10926}{1 - \beta_1^2} - \frac{0.109271}{1 - \beta_2^2} \right) \frac{p_0}{k} \sin \omega t \\ x_2(t) = \left(\psi_{21} \frac{\Delta_{\text{st}}^{(1)}}{1 - \beta_1^2} + \psi_{22} \frac{\Delta_{\text{st}}^{(2)}}{1 - \beta_2^2} \right) \sin \omega t = \left(\frac{1.31575}{1 - \beta_1^2} + \frac{0.184245}{1 - \beta_2^2} \right) \frac{p_0}{k} \sin \omega t \end{cases}$$

Giacomo Boffi

Introductory Remarks

The Homogeneous Problem

Modal Analysis

Examples
2 DOF System