Response by Superposition

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Eigenvector Expansion
Uncoupled Equations of Motion

Eigenvector Expansion

For a N-*DOF* system, it is possible and often advantageous to represent the displacements $\mathbf{x}$ in terms of a linear combination of the free vibration modal shapes, the eigenvectors, by the means of a set of modal coordinates,

$$\mathbf{x} = \sum \boldsymbol{\psi}_i q_i = \boldsymbol{\Psi}\mathbf{q}.$$

The eigenvectors play a role analogous to the role played by trigonometric functions in Fourier Analysis,

- they possess orthogonality properties,
- we will see that it is usually possible to approximate the response using only a few low frequency terms.

Inverting Eigenvector Expansion

The columns of the eigenmatrix $\boldsymbol{\Psi}$ are the N linearly indipendent eigenvectors $\boldsymbol{\psi}_i$, hence the eigenmatrix is non-singular and it is always correct to write $\mathbf{q} = \boldsymbol{\Psi}^{-1}\mathbf{x}$. However, it is not necessary to invert the eigenmatrix: take $\mathbf{x} = \boldsymbol{\Psi}\,\mathbf{q}$, multiply by $\boldsymbol{\Psi}^T\mathbf{M}$ and substitute $\mathbf{M}^\star = \boldsymbol{\Psi}^T\mathbf{M}\boldsymbol{\Psi}$:

$$\boldsymbol{\Psi}^T\mathbf{M}\mathbf{x} = \boldsymbol{\Psi}^T\mathbf{M}\boldsymbol{\Psi}\mathbf{q} = \mathbf{M}^\star\mathbf{q}$$

but $\mathbf{M}^\star$ is a diagonal matrix, hence $(\mathbf{M}^\star)^{-1} = \{\delta_{ij}/M_i\}$ and we can write

$$\mathbf{q} = \mathbf{M}^{\star -1}\boldsymbol{\Psi}^T\mathbf{M}\mathbf{x}, \qquad \text{or} \qquad q_i = \frac{\boldsymbol{\psi}^T\mathbf{M}\mathbf{x}}{M_i}$$

Note: this formula works also when we don't know all the eigenvectors and the inversion of a partial, rectangular $\boldsymbol{\Psi}$ is not feasible.

Undamped System

The displacements and accelerations are written in terms of modal coordinates, $\mathbf{x} = \boldsymbol{\Psi}\,\mathbf{q}$, etc, all terms are premultiplied by the transpose of the eigenvectors matrix, and with $p_i^\star(t) = \boldsymbol{\psi}^T\mathbf{p}(t)$ we can write

$$M_i\,\ddot{q}_i + \omega_i^2 M_i\,q_i = p_i^\star(t), \quad i = 1,\dots,N.$$

The equations of motion written in terms of nodal coordinates constitute a system of N interdipendent, *coupled* differential equations, written in terms of modal coordinates constitute a set of N indipendent, *uncoupled* differential equations.

Damped System

In general,

$$\mathbf{M}\ddot{\mathbf{x}} + \mathbf{C}\dot{\mathbf{x}} + \mathbf{K}\mathbf{x} = \mathbf{p}(t)$$

and with the usual stuff

$$M_i\,\ddot{q}_i + \boldsymbol{\psi}^T\mathbf{C}\boldsymbol{\Psi}\,\dot{\mathbf{q}} + \omega_i^2 M_i q_i = p_i^\star(t),$$

with $\boldsymbol{\psi}_i^T\mathbf{C}\boldsymbol{\psi}_j = c_{ij}$

$$M_i\,\ddot{q}_i + \sum_j c_{ij}\,\dot{q}_j + \omega_i^2 M_i q_i = p_i^\star(t),$$

that is the equations will be uncoupled only if $c_{ij} = \delta_{ij}C_i$.
If we define the damping matrix as

$$\mathbf{C} = \sum_b \mathfrak{c}_b \mathbf{M}\left(\mathbf{M}^{-1}\mathbf{K}\right)^b,$$

we know that, as required,

$$c_{ij} = \delta_{ij}C_i \quad \text{with } C_i\ (= 2\zeta_i M_i \omega_i) = \sum_b \mathfrak{c}_b\left(\omega_i^2\right)^b.$$

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Damped Systems, a Comment

If the response is computed by modal superposition, it is usually preferred a simpler but equivalent procedure: for each mode of interest the analyst imposes a given damping ratio and the integration of the modal equation of equilibrium is carried out as usual.
The $\sum \mathfrak{c}_b$ ... procedure is useful when, e.g. for non-linear problems, the integration of the eq. of motion is carried out in nodal coordinates, because it is easier to specify damping properties globally as elastic modes properties (that can be measured or deduced from similar outsets) than to assign correct damping properties at the *FE* level and assembling $\mathbf{C}$ by the *FEM*.

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Initial Conditions

For a damped system, the modal response can be evaluated, for rest initial conditions, using the Duhamel integral,

$$q_i(t) = \frac{1}{M_i \omega_i} \int_0^t p_i(\tau) e^{-\zeta_i \omega_i (t-\tau)} \sin \omega_{Di}(t-\tau)\, d\tau$$

For different initial conditions $\mathbf{x}_0$, $\dot{\mathbf{x}}_0$, we can easily have the initial conditions in modal coordinates:

$$\mathbf{q}_0 = \mathbf{M}^{\star -1} \boldsymbol{\Psi}^T \mathbf{M} \mathbf{x}_0$$

$$\dot{\mathbf{q}}_0 = \mathbf{M}^{\star -1} \boldsymbol{\Psi}^T \mathbf{M} \dot{\mathbf{x}}_0$$

and the total modal response can be obtained by superposition of Duhamel integral and free vibrations,

$$q_i(t) = e^{-\zeta_i \omega_i t}(q_{i,0} \cos \omega_{Di} t + \frac{\dot{q}_{i,0} + q_{i,0}\zeta_i \omega_i}{\omega_{Di}} \sin \omega_{Di} t) + \cdots$$

Truncated sum

Having computed all $q_i(t)$, we can sum all the modal responses using the eigenvectors,

$$\mathbf{x}(t) = \boldsymbol{\psi}_1 q_1(t) + \boldsymbol{\psi}_2 q_2(t) + \cdots + \boldsymbol{\psi}_N q_N(t) = \sum_{i=1}^{N} \boldsymbol{\psi}_i q_i(t)$$

It is capital to understand that we can use a *truncated sum*, comprising only a few of the lower frequency modes, to obtain a good approximation of structural response:

$$\mathbf{x}(t) \approx \sum_{i=1}^{M<N} \boldsymbol{\psi}_i q_i(t)$$

The importance of truncated sum approximation is twofold:

- less computational effort: less eigenpairs to calculate, less equation of motion to integrate etc
- higher modes (that in common *FEM* models of building structures are rough approximations due to uncertainties in mass distribution details) potentially spurious contributions are excluded from response.

Elastic Forces

Until now, we showed interest in displacements only, but we are interested in elastic forces too. We know that elastic forces can be expressed in terms of displacements and the stiffness matrix:

$$\mathbf{f}_S(t) = \mathbf{K}\mathbf{x}(t) = \mathbf{K}\boldsymbol{\psi}_1 q_1(t) + \mathbf{K}\boldsymbol{\psi}_2 q_2(t) + \cdots .$$

From the characteristic equation we know that

$$\mathbf{K}\boldsymbol{\psi}_i = \omega_i^2 \mathbf{M}\boldsymbol{\psi}_i$$

substituting in the previous equation

$$\mathbf{f}_S(t) = \omega_1^2 \mathbf{M}\boldsymbol{\psi}_1 q_1(t) + \omega_2^2 \mathbf{M}\boldsymbol{\psi}_2 q_2(t) + \cdots .$$

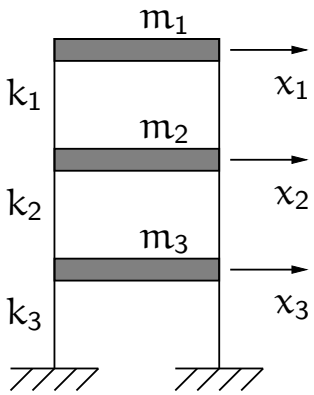
Elastic Forces, 2

Obviously the higher modes' force contributions, e.g.

$$\mathbf{f}_S(t) = \omega_1^2 \mathbf{M}\boldsymbol{\psi}_1 q_1(t) + \cdots + \omega_{20}^2 \mathbf{M}\boldsymbol{\psi}_{20} q_{20}(t) + \cdots$$

in a truncated sum will be higher than displacement ones or, in different words, to estimate internal forces within given accuracy, a greater number of modes must be considered in a truncated sum than the number required to estimate displacements within the same accuracy

Example: problem statement



$$k_1 = 120\,\mathrm{MN/m}, \quad m_1 = 200\,\mathrm{t},$$
$$k_2 = 240\,\mathrm{MN/m}, \quad m_2 = 300\,\mathrm{t},$$
$$k_3 = 360\,\mathrm{MN/m}, \quad m_3 = 400\,\mathrm{t}.$$

1. The above structure is subjected to these initial conditions,

$$\mathbf{x}_0^{\mathsf{T}} = \{5\,\mathrm{mm} \quad 4\,\mathrm{mm} \quad 3\,\mathrm{mm}\},$$
$$\dot{\mathbf{x}}_0^{\mathsf{T}} = \{0 \quad 9\,\mathrm{mm/s} \quad 0\}.$$

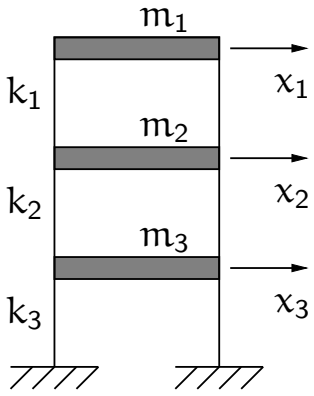
   Write the equation of motion using modal superposition.

2. The above structure is subjected to a half-sine impulse,

$$\mathbf{p}^{\mathsf{T}}(t) = \{1 \quad 2 \quad 2\}\, 2.5\,\mathrm{MN}\, \sin\frac{\pi\, t}{t_1}, \quad \text{with } t_1 = 0.02\,\mathrm{s}.$$

   Write the equation of motion using modal superposition.

Example: structural matrices



$$k_1 = 120\,\mathrm{MN/m}, \quad m_1 = 200\,\mathrm{t},$$
$$k_2 = 240\,\mathrm{MN/m}, \quad m_2 = 300\,\mathrm{t},$$
$$k_3 = 360\,\mathrm{MN/m}, \quad m_3 = 400\,\mathrm{t}.$$

The structural matrices can be written

$$\mathbf{K} = k \begin{bmatrix} 1 & -1 & 0 \\ -1 & 3 & -2 \\ 0 & -2 & 5 \end{bmatrix} = k\overline{\mathbf{K}}, \qquad \text{with } k = 120\frac{\mathrm{MN}}{\mathrm{m}},$$

$$\mathbf{M} = m \begin{bmatrix} 2 & 0 & 0 \\ 0 & 3 & 0 \\ 0 & 0 & 4 \end{bmatrix} = m\overline{\mathbf{M}}, \qquad \text{with } m = 100000\,\mathrm{kg}.$$

Example: adimensional eigenvalues

We want the solutions of the characteristic equation, so we start writing that the determinant of the equation must be zero:

$$\left\| \overline{\mathbf{K}} - \frac{\omega^2}{k/m} \overline{\mathbf{M}} \right\| = \left\| \overline{\mathbf{K}} - \Omega^2 \overline{\mathbf{M}} \right\| = 0,$$

with $\omega^2 = 1200 \left(\frac{\text{rad}}{\text{s}}\right)^2 \Omega^2$.
Expanding the determinant

$$\begin{Vmatrix} 1 - 2\Omega^2 & -1 & 0 \\ -1 & 3 - 3\Omega^2 & -2 \\ 0 & -2 & 5 - 4\Omega^2 \end{Vmatrix} = 0$$

we have the following algebraic equation of 3rd order in Ω^2

$$24 \left(\Omega^6 - \frac{11}{4}\Omega^4 + \frac{15}{8}\Omega^2 - \frac{1}{4} \right) = 0.$$

Example: table of eigenvalues etc

Here are the adimensional roots Ω_i^2, $i = 1, 2, 3$, the dimensional eigenvalues $\omega_i^2 = 1200 \frac{\text{rad}^2}{\text{s}^2} \Omega_i^2$ and all the derived dimensional quantities:

$\Omega_1^2 = 0.17573$	$\Omega_2^2 = 0.8033$	$\Omega_3^2 = 1.7710$
$\omega_1^2 = 210.88$	$\omega_2^2 = 963.96$	$\omega_3^2 = 2125.2$
$\omega_1 = 14.522$	$\omega_2 = 31.048$	$\omega_3 = 46.099$
$f_1 = 2.3112$	$f_2 = 4.9414$	$f_3 = 7.3370$
$T_1 = 0.43268$	$T_3 = 0.20237$	$T_3 = 0.1363$

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Example: eigenvectors and modal matrices

With $\psi_{1j} = 1$, using the 2nd and 3rd equations,

$$\begin{bmatrix} (3 - 3\Omega_j^2) & -2\psi_{3j} \\ -2\psi_{2j} & +(5 - 4\Omega^2 J) \end{bmatrix} \begin{Bmatrix} \psi_{2j} \\ \psi_{3j} \end{Bmatrix} = \begin{Bmatrix} 1 \\ 0 \end{Bmatrix}$$

The above equations must be solved for $j = 1, 2, 3$. The solutions are finally collected in the eigenmatrix

$$\boldsymbol{\Psi} = \begin{bmatrix} 1 & 1 & 1 \\ +0.648535272183 & -0.606599092464 & -2.54193617967 \\ +0.301849953585 & -0.678977475113 & +2.43962752148 \end{bmatrix}.$$

The Modal Matrices are

$$\mathbf{M}^\star = \begin{bmatrix} 362.6 & 0 & 0 \\ 0 & 494.7 & 0 \\ 0 & 0 & 4519.1 \end{bmatrix} \times 10^3 \,\mathrm{kg},$$

$$\mathbf{K}^\star = \begin{bmatrix} 76.50 & 0 & 0 \\ 0 & 477.0 & 0 \\ 0 & 0 & 9603.9 \end{bmatrix} \times 10^6 \,\frac{\mathrm{N}}{\mathrm{m}}$$

Example: initial conditions in modal coordinates

$$\mathbf{q}_0 = (\mathbf{M}^\star)^{-1} \boldsymbol{\Psi}^\mathsf{T} \mathbf{M} \begin{Bmatrix} 5 \\ 4 \\ 3 \end{Bmatrix} \mathrm{mm} = \begin{Bmatrix} +5.9027 \\ -1.0968 \\ +0.1941 \end{Bmatrix} \mathrm{mm},$$

$$\dot{\mathbf{q}}_0 = (\mathbf{M}^\star)^{-1} \boldsymbol{\Psi}^\mathsf{T} \mathbf{M} \begin{Bmatrix} 0 \\ 9 \\ 0 \end{Bmatrix} \frac{\mathrm{mm}}{\mathrm{s}} = \begin{Bmatrix} +4.8288 \\ -3.3101 \\ -1.5187 \end{Bmatrix} \frac{\mathrm{mm}}{\mathrm{s}}$$

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Example: structural response

These are the displacements, in mm

$$x_1 = +5.91\cos(14.5t + .06) + 1.10\cos(31.0t - 3.04) + 0.20\cos(46.1t - 0.17)$$
$$x_2 = +3.83\cos(14.5t + .06) - 0.67\cos(31.0t - 3.04) - 0.50\cos(46.1t - 0.17)$$
$$x_3 = +1.78\cos(14.5t + .06) - 0.75\cos(31.0t - 3.04) + 0.48\cos(46.1t - 0.17)$$

and these the elastic/inertial forces, in kN

$$x_1 = +249.\cos(14.5t + .06) + 212.\cos(31.0t - 3.04) + 084.\cos(46.1t - 0.17)$$
$$x_2 = +243.\cos(14.5t + .06) - 193.\cos(31.0t - 3.04) - 319.\cos(46.1t - 0.17)$$
$$x_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - 0.17)$$

As expected, the contributions of the higher modes are more important for the forces, less important for the displacements.

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Outline

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Introduction

We have a procedure for dynamic analysis of *MDOF* systems based on modal superposition that is both simple and efficient, simple because the modal response can be easily computed with the most advantageous technique that is available for *SDOF* systems, efficient because usually (we will return on this) only the modal responses of a few lower modes are required to accurately describe the structural response.
As the structural matrices are easily assembled using the *FEM*, our modal superposition procedure is ready to be applied to structures with tenth, thousands or millions of *DOF*'s! except that we can compute the eigenpairs only when the analyzed structure has two, three or maybe four degrees of freedom...
Enter the various *Matrix Iterations* procedures!

Equilibrium

First, we will see an iterative procedure whose outputs are the first, or fundamental, mode shape vector and the corresponding eigenvalue.
When an undamped system freely vibrates, the equation of motion is

$$\mathbf{K}\boldsymbol{\psi}_i = \omega_i^2 \mathbf{M}\boldsymbol{\psi}_i.$$

In equilibrium terms, the elastic forces are equal to the inertial forces when the systems oscillates with frequency ω_i^2 and mode shape $\boldsymbol{\psi}_i$

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Proposal of an iterative procedure

Our iterative procedure will be based on finding a new displacement vector $\mathbf{x}_{n+1}$ such that the elastic forces $\mathbf{f}_S = \mathbf{K}\mathbf{x}_{i+1}$ are in equilibrium with the inertial forces due to the *old* displacement vector $\mathbf{x}_n$, $\mathbf{f}_I = \omega_i^2 \mathbf{M}\mathbf{x}_n$.
In equations,

$$\mathbf{K}\mathbf{x}_{n+1} = \omega_i^2 \mathbf{M}\mathbf{x}_n.$$

Premultiplying by the inverse of $\mathbf{K}$ and introducing the *Dynamic Matrix*, $\mathbf{D} = \mathbf{K}^{-1}\mathbf{M}$

$$\mathbf{x}_{n+1} = \omega_i^2 \mathbf{K}^{-1}\mathbf{M}\mathbf{x}_n = \omega_i^2 \mathbf{D}\mathbf{x}_n.$$

It is evident that in the generative equation above we miss a fundamental part, the square of the free vibration frequency ω_i^2.

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The Matrix Iteration Procedure, 1

This problem is solved considering the $\mathbf{x}_n$ as a sequence of *normalized* vectors and introducing the idea of an *unnormalized* new displacement vector, $\hat{\mathbf{x}}_{n+1}$,

$$\hat{\mathbf{x}}_{n+1} = \mathbf{D}\,\mathbf{x}_n,$$

note that we removed the explicit dependency on ω_i^2.
The normalized vector is obtained applying to $\hat{\mathbf{x}}_{n+1}$ a normalizing factor, $\mathfrak{F}_{n+1}$,

$$\mathbf{x}_{n+1} = \frac{\hat{\mathbf{x}}_{n+1}}{\mathfrak{F}_{n+1}},$$

$$\text{but} \qquad \mathbf{x}_{n+1} = \omega_i^2 \mathbf{D}\,\mathbf{x}_n = \omega_i^2\,\hat{\mathbf{x}}_{n+1}, \quad \Rightarrow \quad \frac{1}{\mathfrak{F}} = \omega_i^2$$

If we agree that, near convergence, $\mathbf{x}_{n+1} \approx \mathbf{x}_n$, substituting in the previous equation we have

$$\mathbf{x}_{n+1} \approx \mathbf{x}_n = \omega_i^2\,\hat{\mathbf{x}}_{n+1} \quad \Rightarrow \quad \omega_i^2 \approx \frac{\mathbf{x}_n}{\hat{\mathbf{x}}_{n+1}}.$$

Of course the division of two vectors is not an option, so we want to twist it into something useful.

Normalization

First, consider $\mathbf{x}_n = \boldsymbol{\psi}_i$: in this case, for $j = 1, \dots, N$ it is

$$x_{n,j}/\hat{x}_{n+1,j} = \omega_i^2.$$

Analogously for $\mathbf{x}_n \neq \boldsymbol{\psi}_i$ it was demonstrated that

$$\min_{j=1,\dots,N} \left\{ \frac{x_{n,j}}{\hat{x}_{n+1,j}} \right\} \leqslant \omega_i^2 \leqslant \max_{j=1,\dots,N} \left\{ \frac{x_{n,j}}{\hat{x}_{n+1,j}} \right\}.$$

A more rational approach would make reference to a proper vector norm, so using our preferred vector norm we can write

$$\omega_i^2 \approx \frac{\hat{\mathbf{x}}_{n+1}^{\mathsf{T}} \mathbf{M}\,\mathbf{x}_n}{\hat{\mathbf{x}}_{n+1}^{\mathsf{T}} \mathbf{M}\,\hat{\mathbf{x}}_{n+1}},$$

(if memory helps, this is equivalent to the R_{11} approximation, that we introduced studying Rayleigh quotient refinements).

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Proof of Convergence, 1

Until now we postulated that the sequence $\mathbf{x}_n$ converges to some, unspecified eigenvector $\boldsymbol{\psi}_i$, now we will demonstrate that the sequence converge to the first, or fundamental mode shape,

$$\lim_{n\to\infty} \mathbf{x}_n = \boldsymbol{\psi}_1.$$

Expand $\mathbf{x}_0$ in terms of eigenvectors an modal coordinates:

$$\mathbf{x}_0 = \boldsymbol{\psi}_1 q_{1,0} + \boldsymbol{\psi}_2 q_{2,0} + \boldsymbol{\psi}_3 q_{3,0} + \cdots,$$

and the inertial forces, assuming that the system is vibrating according to the fundamental frequency, are

$$\begin{aligned}\mathbf{f}_{I,n=0} &= \omega_1^2 \mathbf{M}\left(\boldsymbol{\psi}_1 q_{1,0} + \boldsymbol{\psi}_2 q_{2,0} + \boldsymbol{\psi}_3 q_{3,0} + \cdots\right) \\ &= \mathbf{M}\left(\omega_1^2 \boldsymbol{\psi}_1 q_{1,0}\frac{\omega_1^2}{\omega_1^2} + \omega_2^2 \boldsymbol{\psi}_2 q_{2,0}\frac{\omega_1^2}{\omega_2^2} + \cdots\right).\end{aligned}$$

Proof of Convergence, 2

The deflections due to these forces (no hat!, we have multiplied by ω_1^2) are

$$\mathbf{x}_{n=1} = \mathbf{K}^{-1}\mathbf{M}\left(\omega_1^2 \boldsymbol{\psi}_1 q_{1,0}\frac{\omega_1^2}{\omega_1^2} + \omega_2^2 \boldsymbol{\psi}_2 q_{2,0}\frac{\omega_1^2}{\omega_2^2} + \cdots\right),$$

observing that $\omega_j^2 \mathbf{M}\boldsymbol{\psi}_j = \mathbf{K}\boldsymbol{\psi}_j$, substituting and simplifying $\mathbf{K}^{-1}\mathbf{K} = \mathbf{I}$,

$$\mathbf{x}_{n=1} = \left(\boldsymbol{\psi}_1 q_{1,0}\frac{\omega_1^2}{\omega_1^2} + \boldsymbol{\psi}_2 q_{2,0}\frac{\omega_1^2}{\omega_2^2} + \boldsymbol{\psi}_3 q_{3,0}\frac{\omega_1^2}{\omega_3^2} + \cdots\right)$$

applying again this procedure

$$\mathbf{x}_{n=2} = \left(\boldsymbol{\psi}_1 q_{1,0}\left(\frac{\omega_1^2}{\omega_1^2}\right)^2 + \boldsymbol{\psi}_2 q_{2,0}\left(\frac{\omega_1^2}{\omega_2^2}\right)^2 + \boldsymbol{\psi}_3 q_{3,0}\left(\frac{\omega_1^2}{\omega_3^2}\right)^2 + \cdots\right)$$

$$\cdots$$

$$\mathbf{x}_n = \left(\boldsymbol{\psi}_1 q_{1,0}\left(\frac{\omega_1^2}{\omega_1^2}\right)^n + \boldsymbol{\psi}_2 q_{2,0}\left(\frac{\omega_1^2}{\omega_2^2}\right)^n + \boldsymbol{\psi}_3 q_{3,0}\left(\frac{\omega_1^2}{\omega_3^2}\right)^n + \cdots\right)$$

Proof of Convergence, 3

Going to the limit,

$$\lim_{n\to\infty} \mathbf{x}_n = \boldsymbol{\psi}_1 q_{1,0}$$

because

$$\lim_{n\to\infty} \left(\frac{\omega_1^2}{\omega_j^2}\right)^n = \delta_{1j}$$

Consequently,

$$\lim_{n\to\infty} \frac{|\mathbf{x}_n|}{|\hat{\mathbf{x}}_n|} = \omega_1^2$$

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Purified Vectors

If we know $\boldsymbol{\psi}_1$ and ω_1^2 from the matrix iteration procedure it is possible to compute the second eigenpair, following a slightly different procedure.
Express the initial iterate in terms of the (unknown) eigenvectors,

$$\mathbf{x}_{n=0} = \boldsymbol{\Psi}\, \mathbf{q}_{n=0}$$

and premultiply by the (known) $\boldsymbol{\psi}_1^{\mathsf{T}}\mathbf{M}$:

$$\boldsymbol{\psi}_1^{\mathsf{T}}\mathbf{M}\,\mathbf{x}_{n=0} = M_1 q_{1,n=0}$$

solving for $q_{1,n=0}$

$$q_{1,n=0} = \frac{\boldsymbol{\psi}_1^{\mathsf{T}}\mathbf{M}\,\mathbf{x}_{n=0}}{M_1}.$$

Knowing the amplitude of the 1st modal contribution to $\mathbf{x}_{n=0}$ we can write a *purified* vector,

$$\mathbf{y}_{n=0} = \mathbf{x}_{n=0} - \boldsymbol{\psi}_1 q_{1,n=0}.$$

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Convergence (?)

It is easy to demonstrate that using $\mathbf{y}_{n=0}$ as our starting vector

$$\lim_{n\to\infty} \mathbf{y}_n = \boldsymbol{\psi}_2 q_{2,n=0}, \qquad \lim_{n\to\infty} \frac{|\mathbf{y}_n|}{|\hat{\mathbf{y}}_n|} = \omega_2^2.$$

because the initial amplitude of the first mode is null.
Due to numerical errors in the determination of fundamental mode and in the procedure itself, using a plain matrix iteration the procedure however converges to the 1st eigenvector, so to preserve convergence to the 2nd mode it is necessary that the iterated vector $\mathbf{y}_n$ is *purified* at each step n.

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Purification Procedure

The purification procedure is simple, at each step the amplitude of the 1st mode is first computed, then removed from the iterated vector $\mathbf{y}_n$

$$q_{1,n} = \boldsymbol{\psi}_1^{\mathsf{T}} \mathbf{M} \mathbf{y}_n / M_1,$$

$$\hat{\mathbf{y}}_{n+1} = \mathbf{D}\left(\mathbf{y}_n - \boldsymbol{\psi}_1 q_{1,n}\right) = \mathbf{D}\left(\mathbf{I} - \frac{1}{M_1}\boldsymbol{\psi}_1\boldsymbol{\psi}_1^{\mathsf{T}}\mathbf{M}\right)\mathbf{y}_n$$

Introducing the *sweeping matrix* $\mathbf{S}_1 = \mathbf{I} - \frac{1}{M_1}\boldsymbol{\psi}_1\boldsymbol{\psi}_1^{\mathsf{T}}\mathbf{M}$ and the modified dynamic matrix $\mathbf{D}_2 = \mathbf{D}\mathbf{S}_1$, we can write

$$\hat{\mathbf{y}}_{n+1} = \mathbf{D}\mathbf{S}_1\mathbf{y}_n = \mathbf{D}_2\mathbf{y}_n.$$

This is known as *matrix iteration with sweeps*.

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Third Mode

Using again the idea of purifying the iterated vector, starting with the knowledge of the first and the second eigenpair,

$$\hat{\mathbf{y}}_{n+1} = \mathbf{D}\,(\mathbf{y}_n - \boldsymbol{\psi}_1 q_{1,n} - \boldsymbol{\psi}_2 q_{2,n})$$

with $q_{n,1}$ as before and

$$q_{2,n} = \boldsymbol{\psi}_2^T \mathbf{M} \mathbf{y}_n / M_2,$$

substituting in the expression for the purified vector

$$\hat{\mathbf{y}}_{n+1} = \mathbf{D}\big(\underbrace{\mathbf{I} - \frac{1}{M_1}\boldsymbol{\psi}_1\boldsymbol{\psi}_1^T\mathbf{M}}_{\mathbf{S}_1} - \frac{1}{M_2}\boldsymbol{\psi}_2\boldsymbol{\psi}_2^T\mathbf{M}\big)$$

The conclusion is that the sweeping matrix and the modified dynamic matrix to be used to compute the 3rd eigenvector are

$$\mathbf{S}_2 = \mathbf{S}_1 - \frac{1}{M_2}\boldsymbol{\psi}_2\boldsymbol{\psi}_2^T\mathbf{M}, \qquad \mathbf{D}_3 = \mathbf{D}\,\mathbf{S}_2.$$

Generalization to Higher Modes

The results obtained for the third mode are easily generalised.
It is easy to verify that the following procedure can be used to compute all the modes.

Define $\mathbf{S}_0 = \mathbf{I}$, take $i = 1$,

1. compute the modified dynamic matrix to be used for mode i,
$$\mathbf{D}_i = \mathbf{D}\,\mathbf{S}_{i-i}$$
2. compute $\boldsymbol{\psi}_i$ using the modified dynamic matrix;
3. compute the modal mass $M_i = \boldsymbol{\psi}^T\mathbf{M}\,\boldsymbol{\psi}$;
4. compute the sweeping matrix $\mathbf{S}_i$ that *sweeps* the contributions of the first i modes from trial vectors,
$$\mathbf{S}_i = \mathbf{S}_{i-1} - \frac{1}{M_i}\boldsymbol{\psi}_i\boldsymbol{\psi}_i^T\mathbf{M};$$
5. increment i, `GOTO 1`.

Well, we finally have a method that can be used to compute all the eigenpairs of our dynamic problems, full circle!

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Discussion

The method of matrix iteration with sweeping is not used in production because

1. $\mathbf{D}$ is a full matrix, even if $\mathbf{M}$ and $\mathbf{K}$ are banded matrices, and the matrix product that is the essential step in every iteration is computationally onerous,
2. the procedure is however affected by numerical errors,

so, after having demonstrated that it is possible to compute all the eigenvectors of a large problem using an iterative procedure it is time to look for different, more efficient iterative procedures.

Introduction to Inverse Iteration

Inverse iteration is based on the fact that the symmetric stiffness matrix has a banded structure, that is a relatively large triangular portion of the matrix is composed by zeroes (the banded structure is due to the *FEM* model that implies that in an equation of equilibrium the only non zero elastic force coefficients are due to degrees of freedom pertaining to *FE* that contains the degree of freedom for which the equilibrium is written).

Definition of LU decomposition

Every symmetric, banded matrix can be subjected to a so called *LU* decomposition, that is, for $\mathbf{K}$ we write

$$\mathbf{K} = \mathbf{L}\,\mathbf{U}$$

where $\mathbf{L}$ and $\mathbf{U}$ are, respectively, a lower- and an upper-banded matrix.
If we denote with b the bandwidth of $\mathbf{K}$, we have

$$\mathbf{L} = \left[l_{ij}\right] \quad \text{with } l_{ij} \equiv 0 \text{ for } \begin{cases} i < j \\ j < i - b \end{cases}$$

and

$$\mathbf{U} = \left[u_{ij}\right] \quad \text{with } u_{ij} \equiv 0 \text{ for } \begin{cases} i > j \\ j > i + b \end{cases}$$

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Twice the equations?

In this case, with $\boldsymbol{w}_n = \mathbf{M}\,\boldsymbol{x}_n$, the recursion can be written

$$\mathbf{L}\,\mathbf{U}\,\boldsymbol{x}_{n+1} = \boldsymbol{w}_n$$

or as a system of equations,

$$\begin{aligned} \mathbf{U}\,\boldsymbol{x}_{n+1} &= \boldsymbol{z}_{n+1} \\ \mathbf{L}\,\boldsymbol{z}_{n+1} &= \boldsymbol{w}_n \end{aligned}$$

Apparently, we have doubled the number of unknowns, but the z_j's can be easily computed by the procedure of *back substitution*.

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Back Substitution

Temporarily dropping the n and $n+1$ subscripts, we can write

$$
\begin{aligned}
z_1 &= (w_1)/l_{11} \\
z_2 &= (w_2 - l_{21}z_1)/l_{22} \\
z_3 &= (w_3 - l_{31}z_1 - l_{32}z_2)/l_{33} \\
&\dots \\
z_j &= (w_j - \sum_{k=1}^{j-1} l_{jk}z_k)/l_{jj} \\
&\dots
\end{aligned}
$$

The x are then given by $\mathbf{U}x = z$.

Back Substitution

We have computed z by back substitution, we must solve $\mathbf{U}x = z$ but U is upper triangular, so we have

$$
\begin{aligned}
x_N &= (z_N)/u_{NN} \\
x_{N-1} &= (z_{N-1} - u_{N-1,N}z_N)/u_{N-1,N-1} \\
x_{N-2} &= (z_{N-2} - u_{N-2,N}z_N - u_{N-2,N-1}z_{N-1})/u_{N-2,N-2} \\
&\dots \\
x_{N-j} &= (z_{N-j} - \sum_{k=0}^{j-1} u_{N-j,N-k}z_{N-k})/u_{N-j,N-j},
\end{aligned}
$$

For moderately large systems, the reduction in operations count given by back substitution with respect to matrix multiplication is so large that the additional cost of the *LU* decomposition is negligible.

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Introduction to Shifts

Inverse iteration can be applied to each step of matrix iteration with sweeps, or to each step of a different procedure intended to compute all the eigenpairs, the *matrix iteration with shifts*.

Matrix Iteration with Shifts, 1

If we write

$$\omega_i^2 = \mu + \lambda_i,$$

where μ is a *shift* and λ_i is a *shifted eigenvalue*, the eigenvalue problem can be formulated as

$$\mathbf{K}\boldsymbol{\psi}_i = (\mu + \lambda_i)\mathbf{M}\boldsymbol{\psi}_i$$

or

$$(\mathbf{K} - \mu\mathbf{M})\boldsymbol{\psi}_i = \lambda_i \mathbf{M}\boldsymbol{\psi}_i.$$

If we introduce a modified stiffness matrix

$$\overline{\mathbf{K}} = \mathbf{K} - \mu\mathbf{M},$$

we recognize that we have a *new* problem, that has *exactly* the same eigenvectors and *shifted* eigenvalues,

$$\overline{\mathbf{K}}\,\boldsymbol{\phi}_i = \lambda_i \mathbf{M}\boldsymbol{\phi}_i,$$

where

$$\boldsymbol{\phi}_i = \boldsymbol{\psi}_i, \qquad \lambda_i = \omega_i^2 - \mu.$$

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Matrix Iteration with Shifts, 2

The shifted eigenproblem can be solved, e.g., by matrix iteration and the procedure will converge to the *smallest absolute value* shifted eigenvalue and to the associated eigenvector. After convergence is reached,

$$\boldsymbol{\psi}_i = \boldsymbol{\phi}_i, \qquad \omega_i^2 = \lambda_i + \mu.$$

The convergence of the method can be greatly enhanced if the shift μ is updated every few steps during the iterative procedure using the current best estimate of λ_i,

$$\lambda_{i,n+1} = \frac{\hat{\mathbf{x}}_{n+1}\mathbf{M}\,\mathbf{x}_n}{\hat{\mathbf{x}}_{n+1}\mathbf{M}\,\hat{\mathbf{x}}_n},$$

to improve the modified stiffness matric to be used in the following iterations,

$$\overline{\mathbf{K}} = \overline{\mathbf{K}} - \lambda_{i,n+1}\mathbf{M}$$

Much thought was spent on the problem of choosing the initial shifts, so that all the eigenvectors can be computed in sequence without missing any of them.

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Rayleigh Quotient for Discrete Systems

The matrix iteration procedures are usually used in conjunction with methods derived from the Rayleigh Quotient method.
The Rayleigh Quotient method was introduced using distributed flexibilty systems and an assumed shape function, but we have seen also an example where the Rayleigh Quotient was computed for a discrete system using an assumed shape vector.
The procedure to be used for discrete systems can be summarized as

$$\mathbf{x}(t) = \boldsymbol{\phi} Z_0 \sin\omega t, \qquad \dot{\mathbf{x}}(t) = \omega\boldsymbol{\phi} Z_0 \cos\omega t,$$

$$2T_{\max} = \omega^2\boldsymbol{\phi}^{\mathsf{T}}\mathbf{M}\,\boldsymbol{\phi}, \qquad 2V_{\max} = \boldsymbol{\phi}^{\mathsf{T}}\mathbf{K}\,\boldsymbol{\phi},$$

equating the maxima, we have

$$\omega^2 = \frac{\boldsymbol{\phi}^{\mathsf{T}}\mathbf{K}\,\boldsymbol{\phi}}{\boldsymbol{\phi}^{\mathsf{T}}\mathbf{M}\,\boldsymbol{\phi}} = \frac{k^\star}{m^\star},$$

where $\boldsymbol{\phi}$ is an assumed shape vector, not an eigenvector.

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Ritz Coordinates

For a N *DOF* system, an *approximation* to a displacement vector $\boldsymbol{x}$ can be written in terms of a set of $M < N$ assumed shape, linearly independent vectors,

$$\boldsymbol{\Phi}_i, \quad i = 1, \ldots, M < N$$

and a set of *Ritz coordinates* z_i, $i - 1, \ldots, M < N$:

$$\boldsymbol{x} = \sum_i \boldsymbol{\Phi}_i z_i = \boldsymbol{\Phi}\, \boldsymbol{z}.$$

We say *approximation* because a linear combination of $M < N$ vectors cannot describe every point in a N-space.

Rayleigh Quotient in Ritz Coordinates

We can write the Rayleigh quotient as a function of the Ritz coordinates,

$$\omega^2(\boldsymbol{z}) = \frac{\boldsymbol{z}^T \boldsymbol{\Phi}^T \mathbf{K}\, \boldsymbol{\Phi} \boldsymbol{z}}{\boldsymbol{z}^T \boldsymbol{\phi}^T \mathbf{M}\, \boldsymbol{\phi} \boldsymbol{z}} = \frac{\overline{k}(\boldsymbol{z})}{\overline{m}(\boldsymbol{z})},$$

but this is not an explicit fuction for any modal frequency... On the other hand, we have seen that frequency estimates are always greater than true frequencies, so our best estimates are the the local minima of $\omega^2(\boldsymbol{z})$, or the points where all the derivatives of $\omega^2(\boldsymbol{z})$ with respect to z_i are zero:

$$\frac{\partial \omega^2(\boldsymbol{z})}{\partial z_j} = \frac{\overline{m}(\boldsymbol{z}) \dfrac{\partial \overline{k}(\boldsymbol{z})}{\partial z_i} - \overline{k}(\boldsymbol{z}) \dfrac{\partial \overline{m}(\boldsymbol{z})}{\partial z_i}}{(\overline{m}(\boldsymbol{z}))^2} = 0, \qquad \text{for } i = 1, \ldots, M < N$$

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Reduced Eigenproblem

Observing that

$$\overline{k}(z) = \omega^2(z)\overline{m}(z)$$

we can substitute into and simplify the preceding equation,

$$\frac{\partial \overline{k}(z)}{\partial z_i} - \omega^2(z)\frac{\partial \overline{m}(z)}{\partial z_i} = 0, \qquad \text{for } i = 1, \ldots, M < N$$

With the positions

$$\overline{\mathbf{K}} = \mathbf{\Phi}^T \mathbf{K}\, \mathbf{\Phi}, \qquad , \overline{M} = \mathbf{\Phi}^T \mathbf{M}\, \mathbf{\Phi}$$

we have

$$\overline{k}(z) = z^T \overline{\mathbf{K}} z = \sum_i \sum_j \overline{k}_{ij} z_j z_i,$$

and

$$\frac{\partial \overline{k}(z)}{\partial z_i} = 2\sum_j \overline{k}_{ij} z_j = 2\overline{\mathbf{K}}z, \text{ and, analogously, } \frac{\partial \overline{m}(z)}{\partial z_i} = 2\overline{\mathbf{M}}z.$$

Substituting these results in $\frac{\partial \overline{k}(z)}{\partial z_i} - \omega^2(z)\frac{\partial \overline{m}(z)}{\partial z_i} = 0$ we can write a *new eigenvector problem*, in the M *DOF* Ritz coordinates space, with reduced $M \times M$ matrices:

$$\overline{\mathbf{K}}\, z - \omega^2 \overline{\mathbf{M}}\, z = 0.$$

Modal Superposition?

After solving the reduced eigenproblem, we have a set of M eigenvalues $\overline{\omega}_i^2$ and a corresponding set of M eigenvectors $\overline{z}_i$. What is the relation between these results and the eigenpairs of the original problem?
The $\overline{\omega}_i^2$ clearly are approximations from above to the real eigenvalues, and if we write $\overline{\psi}_i = \mathbf{\Phi}\overline{z}_i$ we see that, being

$$\overline{\psi}_i^T \mathbf{M} \overline{\psi}_j = \overline{z}_i^T \underbrace{\mathbf{\Phi}^T \mathbf{M} \mathbf{\Phi}}_{\overline{\mathbf{M}}} \overline{z}_j = \overline{M}_i \delta_{ij},$$

the approximated eigenvectors $\overline{\psi}_i$ are orthogonal with respect to the structural matrices and can be used in ordinary modal superposition techniques.

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A Last Question

One last question: how many $\overline{\omega}_i^2$ and $\overline{\boldsymbol{\Psi}}_i$ are *effective* approximations to the true eigenpairs? Experience tells that an effective approximation is to be expected for the first $M/2$ eigenthings.

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Block Matrix Iteration

If we collect all the eigenvalues into a diagonal matrix $\boldsymbol{\Lambda}$, we can write the following equation,

$$\mathbf{K}\boldsymbol{\Psi} = \mathbf{M}\boldsymbol{\Psi}\boldsymbol{\Lambda}$$

where every matrix is a square, $N \times N$ matrix.
The *Subspace Iteration* method uses a reduced set of trials vectors, packed in $N \times M$ matrix $\boldsymbol{\Phi}_0$ and applies the procedure of matrix iteration to the whole set of trial vectors at once:

$$\hat{\boldsymbol{\Phi}}_1 = \mathbf{K}^{-1}\mathbf{M}\,\boldsymbol{\Phi}_0.$$

We used, again, the hat notation to visualize that the iterated vectors are not normalized by the application of the unknown $\boldsymbol{\Lambda}$.

Should we proceed naively down this road, though, all the columns in $\boldsymbol{\Phi}_n$ would converge to the first eigenvector, subspace iteration being only an expensive manner of applying matrix iteration without sweeps or shifts...

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Different options that comes to mind:

1. force all step n non-normalized vectors to be orthogonal with respect to $\mathbf{M}$, difficult, essentially we have to solve an eigenvalue problem...
2. use the step n non-normalized vectors as a reduced base for the Rayleigh-Ritz procedure, solve an eigenvalue problem

$$\overline{\mathbf{K}}_n = \hat{\mathbf{\Phi}}_n^{\mathsf{T}} \mathbf{K}\, \hat{\mathbf{\Phi}}_n = \hat{\mathbf{\Phi}}_n^{\mathsf{T}} \mathbf{M}\, \mathbf{\Phi}_{n-1}$$

$$\overline{\mathbf{M}}_n = \hat{\mathbf{\Phi}}_n^{\mathsf{T}} \mathbf{M}\, \hat{\mathbf{\Phi}}_n$$

$$\overline{\mathbf{K}}_n\, \overline{\mathbf{Z}}_n = \overline{\mathbf{M}}_n\, \overline{\mathbf{Z}}_n \overline{\mathbf{\Lambda}}_n$$

whose outcome $\overline{\mathbf{\Lambda}}_n$, $\overline{\mathbf{Z}}_n$ *is* correlated to the structural eigenvalues, and use the normalized $\mathbf{Z}_n$ eigenvectors as the normalized, un-hatted $\mathbf{\Phi}_n$.

Subspace Iteration, 2

The second procedure is exactly what we want: we use $\overline{\mathbf{Z}}$ to start an iteration that will lead to a new set of base vectors that, being computed from the equation of dynamic equilibrium, will be a *better* base for the successive estimation of the eigenvectors, a new *subspace* where the eigenvectors can be more closely approximated.

Final Considerations

The procedure converges very fast and with excellent approximation to a number of eigenvalues and eigenvector p, $p = M - q$ where q is the number of required *guard* eigenpairs.
Experience shows that we can safely use $q = \min\{p, 8\}$.

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