

we know that, as required,

$$c_{ij} = \delta_{ij} C_i$$
 with $C_i (= 2\zeta_i M_i \omega_i) = \sum_b \mathfrak{c}_b (\omega_i^2)^b$.

Damped Systems, a Comment

If the response is computed by modal superposition, it is usually preferred a simpler but equivalent procedure: for each mode of interest the analyst imposes a given damping ratio and the integration of the modal equation of equilibrium is carried out as usual.

The $\sum c_b \dots$ procedure is useful when, e.g. for non-linear problems, the integration of the eq. of motion is carried out in nodal coordinates, because it is easier to specify damping properties globally as elastic modes properties (that can be measured or deduced from similar outsets) than to assign correct damping properties at the *FE* level and assembling **C** by the *FEM*.

Truncated sum

Having computed all $q_i(t)$, we can sum all the modal responses,

$$\mathbf{x}(t) = \mathbf{\psi}_1 q_1(t) + \mathbf{\psi}_2 q_2(t) + \dots + \mathbf{\psi}_N q_N(t) = \sum_{i=1}^N \mathbf{\psi}_i q_i(t)$$

It is capital to understand that a *truncated sum*, comprising only a few of the lower frequency modes, gives a good approximation of structural response:

$$\mathbf{x}(t) pprox \sum_{i=1}^{M < N} \boldsymbol{\psi}_i q_i(t)$$

The importance of truncated sum approximation is twofold:

- less computational effort: less eigenpairs to calculate, less equation of motion to integrate etc
- in FEM models the higher modes are rough approximations to structural ones (mostly due to uncertainties in mass distribution details) and the truncated sum excludes potentially spurious contributions from the response.



Elastic Forces, 2

Obviously the higher modes' force contributions, e.g. $% \label{eq:control}$

$$\mathbf{f}_{\mathcal{S}}(t) = \omega_1^2 \mathbf{M} \boldsymbol{\psi}_1 q_1(t) + \dots + \frac{\omega_2^2 \mathbf{M} \boldsymbol{\psi}_2 q_2(t) + \dots$$

in a truncated sum will be higher than displacement ones or, in different words, to estimate internal forces within given accuracy, a greater number of modes must be considered in a truncated sum than the number required to estimate displacements within the same accuracy

Initial Conditions

Superposition

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Superposition

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For a set of generic initial conditions \mathbf{x}_0 , $\dot{\mathbf{x}}_0$, we can easily have the initial conditions in modal coordinates:

$$\mathbf{q}_0 = \mathbf{M}^{\star - 1} \mathbf{\Psi}^T \mathbf{M} \mathbf{x}_0$$

$$\dot{\mathbf{q}}_0 = \mathbf{M}^{\star - 1} \mathbf{\Psi}^T \mathbf{M} \dot{\mathbf{x}}_0$$

and, for each mode, the total modal response can be obtained by superposition of a particular integral $\xi_i(t)$ and the general integral of the homogeneous associate,

$$q_{i}(t) = e^{-\zeta_{i}\omega_{i}t} \times ($$

$$(q_{i,0} - \xi_{i}(0)) \cos \omega_{Di}t +$$

$$+ \frac{(\dot{q}_{i,0} - \dot{\xi}_{i}(0)) + (q_{i,0} - \xi_{i}(0))\zeta_{i}\omega_{i}}{\omega_{Di}} \sin \omega_{Di}t$$

$$) + \xi_{i}(t)$$

Elastic Forces

Until now, we showed interest in displacements only, but we are interested in elastic forces too. We know that elastic forces can be expressed in terms of displacements and the stiffness matrix:

$$\mathbf{f}_{S}(t) = \mathbf{K} \mathbf{x}(t) = \mathbf{K} \boldsymbol{\psi}_{1} q_{1}(t) + \mathbf{K} \boldsymbol{\psi}_{2} q_{2}(t) + \cdots$$

From the characteristic equation we know that

$$\mathbf{K}\boldsymbol{\psi}_i = \omega_i^2 \mathbf{M}\boldsymbol{\psi}_i$$

substituting in the previous equation

$$\mathbf{f}_{S}(t) = \boldsymbol{\omega}_{1}^{2} \mathbf{M} \boldsymbol{\psi}_{1} q_{1}(t) + \boldsymbol{\omega}_{2}^{2} \mathbf{M} \boldsymbol{\psi}_{2} q_{2}(t) + \cdots$$

SuperpositionGacomo BoffiEigenvector
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$$m_1$$

 k_2
 k_3 $k_1 = 120 \text{ MN/m}, m_1 = 200 \text{ t}, m_2$
 $k_2 = 240 \text{ MN/m}, m_2 = 300 \text{ t}, m_2$
 $k_3 = 360 \text{ MN/m}, m_3 = 400 \text{ t}.$ Uncoupled
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Europe1. The above structure is subjected to these initial conditions,
 $\mathbf{x}_0^T = \{5 \text{ mm} 4 \text{ mm} 3 \text{ mm}\},$
 $\mathbf{x}_0^T = \{0 \ 9 \text{ mm/s} \ 0\}$.
Write the equation of motion using modal superposition.2. The above structure is subjected to a half-sine impulse,
 $\mathbf{p}^T(t) = \{1 \ 2 \ 2\} 2.5 \text{ MN sin} \frac{\pi t}{t_1}, \text{ with } t_1 = 0.02 \text{ s}.$
Write the equation of motion using modal superposition.

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Superposition

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Example: structural matrices

$$\begin{array}{c}
 m_1 \\
 k_1 \\
 m_2 \\
 k_2 \\
 m_3 \\
 k_2 \\
 k_3 \\
 m_3 \\
 m_4 \\
 m_4 \\
 m_4 \\
 m_4 \\
 m_5 \\$$

Example

 $\Psi =$

Example: adimensional eigenvalues

We want the solutions of the characteristic equation, so we start writing that the determinant of the equation must be zero:

$$\left\|\overline{\mathbf{K}} - \frac{\omega^2}{k/m}\overline{\mathbf{M}}\right\| = \left\|\overline{\mathbf{K}} - \Omega^2\overline{\mathbf{M}}\right\| = 0,$$

with $\omega^2 = 1200 \left(\frac{\text{rad}}{\text{s}}\right)^2 \Omega^2$. Expanding the determinant

$$\begin{vmatrix} 1 - 2\Omega^2 & -1 & 0 \\ -1 & 3 - 3\Omega^2 & -2 \\ 0 & -2 & 5 - 4\Omega^2 \end{vmatrix} = 0$$

we have the following algebraic equation of 3rd order in Ω^2

$$24\left(\Omega^6 - \frac{11}{4}\Omega^4 + \frac{15}{8}\Omega^2 - \frac{1}{4}\right) = 0.$$

Example: table of eigenvalues etc	Superposition Giacomo Boffi	Example: eigen
Here are the adimensional roots Ω_i^2 , $i = 1, 2, 3$, the dimensional eigenvalues $\omega_i^2 = 1200 \frac{\text{rad}^2}{\text{s}^2} \Omega_i^2$ and all the derived dimensional quantities: $\Omega_1^2 = 0.17573$ $\Omega_2^2 = 0.8033$ $\Omega_3^2 = 1.7710$ $\omega_1^2 = 210.88$ $\omega_2^2 = 963.96$ $\omega_3^2 = 2125.2$ $\omega_1 = 14.522$ $\omega_2 = 31.048$ $\omega_3 = 46.099$ $f_1 = 2.3112$ $f_2 = 4.9414$ $f_3 = 7.3370$ $T_1 = 0.43268$ $T_3 = 0.20237$ $T_3 = 0.1363$	Eigenvector Expansion Uncoupled Equations of Motion Undamped Dumped System Tractard Sam Elastic Fores Eampe	With $\psi_{1j} = 1$, using th $\begin{bmatrix} 3 & - \\ - \\ - \end{bmatrix}$ The above equations in For $j = 1$, it is $\begin{cases} 2.47280290 \\ -2\psi_2 \end{cases}$ For $j = 2$, $\begin{cases} 0.59010136 \\ -2\psi_2 \end{cases}$ Finally, for $j = 3$, $\begin{cases} -2.3129042i \\ -2\psi_2 \end{cases}$

Example: eigenvectors and modal matrices	Superposition
With $\psi_{1j} = 1$, using the 2nd and 3rd equations, $ \begin{bmatrix} 3 - 3\Omega_j^2 & -2 \\ -2 & 5 - 4\Omega_j^2 \end{bmatrix} \left\{ \psi_{2j} \\ \psi_{3j} \right\} = \begin{cases} 1 \\ 0 \end{cases} $	Giacomo Boffi Eigenvector Expansion Uncoupled Equations of Motion
The above equations must be solved for $j = 1, 2, 3$. For $j = 1$, it is	Undamped Damped System Truncated Sum Elastic Forces Example
$\begin{cases} 2.47280290827\psi_{21} & -2\psi_{31} & = & 1 \\ -2\psi_{21} & +4.29707054436\psi_{31} & = & 0 \end{cases}$	
For $j = 2$,	
$\begin{cases} 0.5901013613\psi_{22} & -2\psi_{32} & = 1 \\ -2\psi_{22} & +1.78680181507\psi_{32} & = 0 \end{cases}$	
Finally, for $j = 3$,	
$\begin{cases} -2.31290426958\psi_{23} & -2\psi_{33} &= 1\\ -2\psi_{23} & -2.08387235944\psi_{33} &= 0 \end{cases}$	

Superposition Superposition Example: initial conditions in modal Giacomo Boffi Giacomo Boff coordinates The solutions are finally collected in the eigenmatrix $\begin{bmatrix} 1 & 1 & 1 \\ +0.648535272183 & -0.606599092464 & -2.54193617967 \\ +0.301849953585 & -0.678977475113 & +2.43962752148 \end{bmatrix}$ $\mathbf{q}_{0} = (\mathbf{M}^{\star})^{-1} \mathbf{\Psi}^{\top} \mathbf{M} \begin{cases} 5\\ 4\\ 3 \end{cases} \text{ mm} = \begin{cases} +5.9027\\ -1.0968\\ +0.1941 \end{cases} \text{ mm},$ The Modal Matrices are $\mathbf{M}^{*} = \mathbf{\Psi}^{T} \mathbf{M} \, \mathbf{\Psi} = \begin{bmatrix} 362.6 & 0 & 0 \\ 0 & 494.7 & 0 \\ 0 & 0 & 4519.1 \end{bmatrix} \times 10^{3} \, \text{kg},$ $\mathbf{K}^{*} = \mathbf{\Psi}^{T} \mathbf{K} \, \mathbf{\Psi} = \begin{bmatrix} 76.50 & 0 & 0 \\ 0 & 477.0 & 0 \\ 0 & 0 & 9603.9 \end{bmatrix} \times 10^{6} \frac{\text{N}}{\text{m}}$ $\dot{\mathbf{q}}_0 = (\mathbf{M}^{\star})^{-1} \boldsymbol{\Psi}^{\mathsf{T}} \mathbf{M} \begin{cases} 0\\ 9\\ 0 \end{cases} \frac{\mathsf{mm}}{\mathsf{s}} = \begin{cases} +4.8288\\ -3.3101\\ -1.5187 \end{cases} \frac{\mathsf{mm}}{\mathsf{s}}$

Superposition Giacomo Boff Uncoupled Equations of Aotion Indamped Iamped System nuncated Sum astic Forces ample

Example: structural response These are the displacements, in mm $x_1 = +5.91 \cos(14.5t + .06) + 1.10 \cos(31.0t - 3.04) + 0.20 \cos(46.1t - x_2 = +3.83 \cos(14.5t + .06) - 0.67 \cos(31.0t - 3.04) - 0.50 \cos(46.1t - x_3 = +1.78 \cos(14.5t + .06) - 0.75 \cos(31.0t - 3.04) + 0.48 \cos(46.1t - and these the elastic/inertial forces, in kN f_1 = +249.\cos(14.5t + .06) + 212.\cos(31.0t - 3.04) + 084.\cos(46.1t - f_2 = +243.\cos(14.5t + .06) - 193.\cos(31.0t - 3.04) + 084.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 = +151.\cos(14.5t + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 + .06) - 288.\cos(31.0t - 3.04) + 408.\cos(46.1t - f_3 + .06) - 480.\cos(46.1t - 60.5t + .06) - 480.5t + .06)$	Danjed System - 0.17) d Sum Faratic Forces - 0.17) - 0.17)	Matrix Iteration Giacomo Boffi Dipartimento di Ingegneria Civile e Ambientale, Politecnico di Milano May 13, 2013	Matrix Iteration Giacomo Boffi Introduction Fundamental Mode Analysis Second Mode Analysis Higher Modes Inverse Iteration Matrix Iteration with Shifts Alternative Procedures
Outline	Matrix Iteration	Introduction	Matrix Iteration
Introduction Fundamental Mode Analysis Second Mode Analysis Higher Modes Inverse Iteration Matrix Iteration with Shifts Alternative Procedures Rayleigh Quotient Rayleigh-Ritz Method Subspace Iteration	Giacomo Boffi Introduction Fundamental Mode Analysis Second Mode Analysis Higher Modes Inverse Iteration Matrix Iteration with Shifts Alternative Procedures	 Dynamic analysis of <i>MDOF</i> systems based on modal superposition is both simple and efficient simple: the modal response can be easily computed, analitically or numerically, with the techniques we have seen for <i>SDOF</i> systems, efficient: in most cases, only the modal responses of a few lower modes are required to accurately describe the structural response. As the structural matrices are easily assembled using the <i>FEM</i>, our modal superposition procedure is ready to be applied to structures with tenth, thousands or millions of <i>DOF</i>'s! except that we can compute the eigenpairs only when the analyzed structure has two, three or maybe four degrees of freedom Enter the various <i>Matrix Iterations</i> procedures! 	Giacomo Boffi Introduction Fundamental Mode Analysis Second Modes Second Modes Inverse Iteration Matrix Iteration with Shifts Procedures
Equilibrium First, we will see an iterative procedure whose outputs are the first, or fundamental, mode shape vector and the corresponding eigenvalue. When an undamped system freely vibrates, the equation of motion is $\mathbf{K} \boldsymbol{\psi}_i = \omega_i^2 \mathbf{M} \boldsymbol{\psi}_i.$ In equilibrium terms, the elastic forces are equal to the inertial forces when the systems oscillates with frequency ω_i^2 and mode shape $\boldsymbol{\psi}_i$	Matrix Iteration Giacomo Boffi Introduction Fundamental Mode Analysis Second Mode Analysis Higher Modes Inverse Iteration Matrix Iteration with Shifts Alternative Procedures	Proposal of an iterative procedure Our iterative procedure will be based on finding a new displacement vector \mathbf{x}_{n+1} such that the elastic forces $\mathbf{f}_{S} = \mathbf{K} \mathbf{x}_{n+1}$ are in equilibrium with the inertial forces due to the old displacement vector \mathbf{x}_{n} , $\mathbf{f}_{l} = \omega_{i}^{2} \mathbf{M} \mathbf{x}_{n}$, $\mathbf{K} \mathbf{x}_{n+1} = \omega_{i}^{2} \mathbf{M} \mathbf{x}_{n}$. Premultiplying by the inverse of \mathbf{K} and introducing the Dynamic Matrix, $\mathbf{D} = \mathbf{K}^{-1}\mathbf{M}$ $\mathbf{x}_{n+1} = \omega_{i}^{2}\mathbf{K}^{-1}\mathbf{M}\mathbf{x}_{n} = \omega_{i}^{2}\mathbf{D}\mathbf{x}_{n}$. In the generative equation above we miss a fundamental part, the square of the free vibration frequency ω_{i}^{2} .	Matrix Iteration Giacomo Boffi Introduction Fundamental Mode Analysis Second Mode Analysis Higher Modes Inverse Iteration Watrix Iteration Watrix Iteration With Shifts Alternative Procedures

The Matrix Iteration Procedure, 1

This problem is solved considering the \mathbf{x}_n as a sequence of *normalized* vectors and introducing the idea of an *unnormalized* new displacement vector, $\hat{\mathbf{x}}_{n+1}$, $\hat{\mathbf{x}}_{n+1} = \mathbf{D} \mathbf{x}_n$

note that we removed the explicit dependency on
$$\omega_i^2$$
.
The normalized vector is obtained applying to $\hat{\mathbf{x}}_{n+1}$ a normalizing factor, \mathfrak{F}_{n+1} ,

$$\mathbf{x}_{n+1} = rac{\hat{\mathbf{x}}_{n+1}}{\mathfrak{F}_{n+1}},$$

but
$$\mathbf{x}_{n+1} = \omega_i^2 \mathbf{D} \mathbf{x}_n = \omega_i^2 \, \hat{\mathbf{x}}_{n+1}, \quad \Rightarrow \quad \frac{1}{\mathfrak{F}} = \omega_i^2$$

If we agree that, near convergence, $\mathbf{x}_{n+1}\approx\mathbf{x}_n,$ substituting in the previous equation we have

$$\mathbf{x}_{n+1} \approx \mathbf{x}_n = \omega_i^2 \, \hat{\mathbf{x}}_{n+1} \quad \Rightarrow \quad \omega_i^2 \approx \frac{\mathbf{x}_n}{\hat{\mathbf{x}}_{n+1}}.$$

Of course the division of two vectors is not an option, so we want to twist it into something useful.

Proof of Convergence, 1

Until now we postulated that the sequence \mathbf{x}_n converges to some, unspecified eigenvector $\boldsymbol{\psi}_i$, now we will demonstrate that the sequence converge to the first, or fundamental mode shape,

$$\lim_{n\to\infty}\mathbf{x}_n=\boldsymbol{\psi}_1.$$

1. Expand \boldsymbol{x}_0 in terms of eigenvectors and modal coordinates:

$$\mathbf{x}_0 = \mathbf{\psi}_1 q_{1,0} + \mathbf{\psi}_2 q_{2,0} + \mathbf{\psi}_3 q_{3,0} + \cdot$$

2. The inertial forces, assuming that the system is vibrating according to the fundamental frequency, are

$$\mathbf{f}_{I,n=0} = \omega_1^2 \mathbf{M} \left(\boldsymbol{\psi}_1 q_{1,0} + \boldsymbol{\psi}_2 q_{2,0} + \boldsymbol{\psi}_3 q_{3,0} + \cdots \right) \\ = \mathbf{M} \left(\omega_1^2 \boldsymbol{\psi}_1 q_{1,0} \frac{\omega_1^2}{\omega_1^2} + \omega_2^2 \boldsymbol{\psi}_2 q_{2,0} \frac{\omega_1^2}{\omega_2^2} + \cdots \right).$$

Proof of Convergence, 3

5. Applying again the previous procedure, i.e., premultiply the right member by $\omega_1^2 \mathbf{D}$, multiplying and dividing each term by ω_i^2 , symplifying, we have

$$\mathbf{x}_{n=2} = \boldsymbol{\psi}_1 q_{1,0} \left(\frac{\omega_1^2}{\omega_1^2}\right)^2 + \boldsymbol{\psi}_2 q_{2,0} \left(\frac{\omega_1^2}{\omega_2^2}\right)^2 + \boldsymbol{\psi}_3 q_{3,0} \left(\frac{\omega_1^2}{\omega_3^2}\right)^2 + \cdots$$

6. repeating the procedure for *n* times starting from \mathbf{x}_0 , we have

$$\mathbf{x}_{n} = \boldsymbol{\psi}_{1} q_{1,0} \left(\frac{\omega_{1}^{2}}{\omega_{1}^{2}}\right)^{n} + \boldsymbol{\psi}_{2} q_{2,0} \left(\frac{\omega_{1}^{2}}{\omega_{2}^{2}}\right)^{n} + \boldsymbol{\psi}_{3} q_{3,0} \left(\frac{\omega_{1}^{2}}{\omega_{3}^{2}}\right)^{n} + \cdot$$

Matrix Iteration

Second Mode Analysis Higher Modes Inverse Iteration Matrix Iteration

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Normalization

Matrix Iteration

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Fundamental Mode Analysis

Matrix Iteration

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Fundamental Mode Analysis First, consider $\mathbf{x}_n = \boldsymbol{\psi}_j$: in this case, for j = 1, ..., N it is

$$x_{n,j}/\hat{x}_{n+1,j}=\omega_i^2.$$

Analogously for $\mathbf{x}_n \neq \boldsymbol{\psi}_i$ it was demonstrated that

$$\min_{j=1,\ldots,N}\left\{\frac{x_{n,j}}{\hat{x}_{n+1,j}}\right\} \le \omega_i^2 \le \max_{j=1,\ldots,N}\left\{\frac{x_{n,j}}{\hat{x}_{n+1,j}}\right\}.$$

A more rational approach would make reference to a proper vector norm, so using our preferred vector norm we can write

$$\omega_i^2 \approx \frac{\hat{\mathbf{x}}_{n+1}^{\prime} \mathbf{M} \mathbf{x}_n}{\hat{\mathbf{x}}_{n+1}^{T} \mathbf{M} \hat{\mathbf{x}}_{n+1}}$$

(if memory helps, this is equivalent to the R_{11} approximation, that we introduced studying Rayleigh quotient refinements).

Proof of Convergence, 2

3. The deflections due to these forces (no hat!, we have multiplied by $\omega_1^2)$ are

$$\mathbf{x}_{n=1} = \mathbf{K}^{-1} \mathbf{M} \left(\omega_1^2 \boldsymbol{\psi}_1 q_{1,0} \frac{\omega_1^2}{\omega_1^2} + \omega_2^2 \boldsymbol{\psi}_2 q_{2,0} \frac{\omega_1^2}{\omega_2^2} + \cdots \right),$$

(note that we have multiplied and divided each term by ω_i^2). 4. Using $\omega_i^2 \mathbf{M} \, \boldsymbol{\psi}_i = \mathbf{K} \, \boldsymbol{\psi}_i$,

$$\begin{aligned} \mathbf{x}_{n=1} &= \mathbf{K}^{-1} \left(\mathbf{K} \boldsymbol{\psi}_1 q_{1,0} \frac{\omega_1^2}{\omega_1^2} + \mathbf{K} \boldsymbol{\psi}_2 q_{2,0} \frac{\omega_1^2}{\omega_2^2} + \mathbf{K} \boldsymbol{\psi}_3 q_{3,0} \frac{\omega_1^2}{\omega_3^2} + \cdots \right. \\ &= \boldsymbol{\psi}_1 q_{1,0} \frac{\omega_1^2}{\omega_1^2} + \boldsymbol{\psi}_2 q_{2,0} \frac{\omega_1^2}{\omega_2^2} + \boldsymbol{\psi}_3 q_{3,0} \frac{\omega_1^2}{\omega_3^2} + \cdots \end{aligned}$$

Introduction Fundamental Mode Analysis Second Mode Analysis Higher Modes Inverse Iteration Matrix Iteration

Iternative

Matrix Iteration

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Fundamental Mode Analysis

Going to the limit,

Proof of Convergence, 4

$$\lim_{n\to\infty}\mathbf{x}_n=\boldsymbol{\psi}_1q_{1,0}$$

because

Consequently,

$$\lim_{n\to\infty}\left(\frac{\omega_1^2}{\omega_j^2}\right)^n =$$

$$\lim_{n \to \infty} \frac{|\mathbf{x}_n|}{|\hat{\mathbf{x}}_n|} = \omega_1^2$$

 $=\delta_{1j}$

Matrix Iteration

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Fundamental Mode Analysis

Matrix Iteration **Purified Vectors** Giacomo Boffi

If we know $\boldsymbol{\psi}_1$ and ω_1^2 from the matrix iteration procedure it is possible to compute the second eigenpair, following a slightly different procedure. Express the initial iterate in terms of the (unknown)

eigenvectors, $\mathbf{x}_{n=0} = \mathbf{\Psi} \mathbf{q}_{n=0}$

and premultiply by the (known) $\boldsymbol{\psi}_1^T \mathbf{M}$:

 $\boldsymbol{\psi}_1^T \mathbf{M} \mathbf{x}_{n=0} = M_1 q_{1,n=0}$

solving for $q_{1,n=0}$

$$q_{1,n=0} = \frac{\boldsymbol{\psi}_1^T \mathbf{M} \mathbf{x}_{n=0}}{M_1}$$

Knowing the amplitude of the 1st modal contribution to $\mathbf{x}_{n=0}$ we can write a *purified* vector,

$$\mathbf{y}_{n=0} = \mathbf{x}_{n=0} - \mathbf{\psi}_1 q_{1,n=0}.$$

Purification Procedure

The purification procedure is simple, at each step the amplitude of the 1st mode is first computed, then removed from the iterated vector \mathbf{y}_n

 $a_{1,n} = \boldsymbol{\psi}_1^T \mathbf{M} \mathbf{v}_n / M_1$

$$\hat{\boldsymbol{y}}_{n+1} = \boldsymbol{\mathsf{D}} \left(\boldsymbol{\mathsf{y}}_n - \boldsymbol{\psi}_1 q_{1,n} \right) = \boldsymbol{\mathsf{D}} \left(\boldsymbol{\mathsf{I}} - \frac{1}{M_1} \boldsymbol{\psi}_1 \boldsymbol{\psi}_1^T \boldsymbol{\mathsf{M}} \right) \boldsymbol{\mathsf{y}}_n$$

Introducing the sweeping matrix $\mathbf{S}_1 = \mathbf{I} - \frac{1}{M_1} \boldsymbol{\psi}_1 \boldsymbol{\psi}_1^\top \mathbf{M}$ and the modified dynamic matrix $\mathbf{D}_2 = \mathbf{DS}_1$, we can write

 $\hat{\mathbf{y}}_{n+1} = \mathbf{D}\mathbf{S}_1\mathbf{y}_n = \mathbf{D}_2\mathbf{y}_n.$

This is known as matrix iteration with sweeps.

Generalization to Higher Modes

The results obtained for the third mode are easily generalised. It is easy to verify that the following procedure can be used to compute all the modes.

Define $\mathbf{S}_0 = \mathbf{I}$, let i = 1,

1. compute the modified dynamic matrix to be used for mode *i*,

$$\mathbf{D}_i = \mathbf{D} \, \mathbf{S}_{i-i}$$

- 2. compute $\boldsymbol{\psi}_i$ using the modified dynamic matrix;
- 3. compute the modal mass $M_i = \boldsymbol{\psi}^T \mathbf{M} \boldsymbol{\psi}$;
- 4. compute the sweeping matrix **S**_i that sweeps the contributions of the first *i* modes from trial vectors,

$$\mathbf{S}_i = \mathbf{S}_{i-1} - \frac{1}{M_i} \boldsymbol{\psi}_i \boldsymbol{\psi}_i^T \mathbf{M};$$

5 increment *i* GOTO 1

Well, we finally have a method that can be used to compute all the eigenpairs of our dynamic problems, full circle!

Convergence (?)

It is easy to demonstrate that using $\mathbf{y}_{n=0}$ as our starting vector

$$\lim_{n\to\infty}\mathbf{y}_n = \boldsymbol{\psi}_2 q_{2,n=0}, \qquad \lim_{n\to\infty} \frac{|\mathbf{y}_n|}{|\mathbf{\hat{y}}_n|} = \omega_2^2.$$

because the initial amplitude of the first mode is null. Due to numerical errors in the determination of fundamental mode and in the procedure itself, using a plain matrix iteration the procedure however converges to the 1st eigenvector, so to preserve convergence to the 2nd mode it is necessary that the iterated vector \mathbf{y}_n is *purified* at each step n.

Third Mode

Using again the idea of purifying the iterated vector, starting with the knowledge of the first and the second eigenpair,

$$\hat{\mathbf{y}}_{n+1} = \mathbf{D} \left(\mathbf{y}_n - \boldsymbol{\psi}_1 q_{1,n} - \boldsymbol{\psi}_2 q_{2,n} \right)$$

with $q_{n,1}$ as before and

$$q_{2,n} = \boldsymbol{\psi}_2^{T} \, \mathbf{M} \mathbf{y}_n / M_2$$

substituting in the expression for the purified vector

$$\hat{\mathbf{y}}_{n+1} = \mathbf{D} \big(\underbrace{\mathbf{I} - \frac{1}{M_1} \boldsymbol{\psi}_1 \boldsymbol{\psi}_1^{\mathsf{T}} \mathbf{M}}_{\mathbf{S}_1} - \frac{1}{M_2} \boldsymbol{\psi}_2 \boldsymbol{\psi}_2^{\mathsf{T}} \mathbf{M} \big)$$

The conclusion is that the sweeping matrix and the modified dynamic matrix to be used to compute the 3rd eigenvector are

$$\mathbf{S}_2 = \mathbf{S}_1 - \frac{1}{M_2} \boldsymbol{\psi}_2 \boldsymbol{\psi}_2^T \mathbf{M}, \qquad \mathbf{D}_3 = \mathbf{D} \, \mathbf{S}_2.$$

Discussion

The method of matrix iteration with sweeping is not used in production because

1. $\boldsymbol{\mathsf{D}}$ is a full matrix, even if $\boldsymbol{\mathsf{M}}$ and $\boldsymbol{\mathsf{K}}$ are banded matrices, and the matrix product that is the essential step in every iteration is computationally onerous,

2. the procedure is however affected by numerical errors. While it is possible to compute all the eigenvectors and eigenvalues of a large problem using our iterative procedure, we can first optimize our procedure and later seek for different, more efficient iterative procedures.

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Higher Modes

Matrix Iteration

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Higher Modes

Second Mode Analysis

Matrix Iteration

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Higher Modes

Matrix Iteration

Second Mode Analysis

Inverse iteration to Inverse Iteration Inverse iteration is based on the fact that the symmetric stiffness matrix has a banded structure, that is a relatively large triangular portion of the matrix is composed by zeroes The banded structure is due to the <i>FEM</i> model that implies that in an equation of equilibrium the only non zero elastic force coefficients are due to degrees of freedom pertaining to <i>FE</i> that contains the degree of freedom for which the equilibrium is written).	Matrix Iteration Giacomo Boffi Introduction Fundamental Mode Analysis Second Mode Analysis Second Mode Analysis Higher Modes Inverse Iteration with Shifts Alternative Procedures	Definition of <i>LU</i> decomposition Every symmetric, banded matrix can be subjected to a so called <i>LU</i> decomposition, that is, for K we write $\mathbf{K} = \mathbf{L}\mathbf{U}$ where L and U are, respectively, a lower- and an upper-banded matrix. If we denote with <i>b</i> the bandwidth of K , we have $\mathbf{L} = \begin{bmatrix} I_{ij} \end{bmatrix} \text{ with } I_{ij} \equiv 0 \text{ for } \begin{cases} i < j \\ j < i - b \end{cases}$ and $\mathbf{U} = \begin{bmatrix} u_{ij} \end{bmatrix} \text{ with } u_{ij} \equiv 0 \text{ for } \begin{cases} i > j \\ j > i + b \end{cases}$	Matrix Iteration Giacomo Boffi Introduction Fundamental Mode Analysis Second Mode Analysis Higher Modes Inverse Iteration Matrix Iteration With Shifts Alternative Procedures
Twice the equations? In this case, with $\mathbf{w}_n = \mathbf{M} \mathbf{x}_n$, the recursion can be written	Matrix Iteration Giacomo Boffi Introduction Fundamental Mode Analysis Second Mode	Back Substitution Temporarily dropping the n and $n + 1$ subscripts, we can write	Matrix Iteration Giacomo Boffi Introduction Fundamental Mode Analysis Second Mode

Back Substitution

substitution.

or as a system of equations,

We have computed \mathbf{z} by back substitution, we must solve $\mathbf{U}\mathbf{x} = \mathbf{z}$ but U is upper triangular, so we have

 $LUx_{n+1} = w_n$

 $\mathbf{U} \mathbf{x}_{n+1} = \mathbf{z}_{n+1}$

 $L z_{n+1} = w_n$ Apparently, we have doubled the number of unknowns, but the z_j 's can be easily computed by the procedure of back

$$x_{N} = (z_{N})/u_{NN}$$

$$x_{N-1} = (z_{N-1} - u_{N-1,N}z_{N})/u_{N-1,N-1}$$

$$x_{N-2} = (z_{N-2} - u_{N-2,N}z_{N} - u_{N-2,N-1}z_{N-1})/u_{N-2,N-2}$$

...

$$j-1$$

 $x_{N-j} = (z_{N-j} - \sum_{k=0}^{j-1} u_{N-j,N-k} z_{N-k})/u_{N-j,N-j},$

For moderately large systems, the reduction in operations count given by back substitution with respect to matrix multiplication is so large that the additional cost of the LUdecomposition is negligible.

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Inverse Iteration

Inverse Iteration

$$z_1 = (w_1)/l_{11}$$

$$z_2 = (w_2 - l_{21}z_1)/l_{22}$$

$$z_3 = (w_3 - l_{31}z_1 - l_{32}z_2)/l_{33}$$

...

Inverse Iteration

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,

$$z_j = (w_j - \sum_{k=1}^{j-1} l_{jk} z_k) / l_{jj}$$
...

The **x** are then given by $\mathbf{U}\mathbf{x} = \mathbf{z}$.

Introduction to Shifts

Inverse iteration can be applied to each step of matrix Matrix Iteration with Shifts iteration with sweeps, or to each step of a different procedure intended to compute all the eigenpairs, the matrix iteration with shifts.

Matrix Iteration with Shifts, 1

If we write

 $\omega_i^2 = \mu + \lambda_i,$

where μ is a *shift* and λ_i is a *shifted eigenvalue*, the eigenvalue problem can be formulated as

 $\mathbf{K} \, \boldsymbol{\psi}_i = (\mu + \lambda_i) \mathbf{M} \, \boldsymbol{\psi}_i$

or

 $(\mathbf{K} - \mu \mathbf{M}) \boldsymbol{\psi}_i = \lambda_i \mathbf{M} \, \boldsymbol{\psi}_i.$

If we introduce a modified stiffness matrix

 $\overline{\mathbf{K}} = \mathbf{K} - \mu \mathbf{M}$.

we recognize that we have a *new* problem, that has *exactly* the same eigenvectors and *shifted* eigenvalues,

 $\overline{\mathbf{K}} \boldsymbol{\phi}_i = \lambda_i \mathbf{M} \boldsymbol{\phi}_i,$

where

$$\boldsymbol{\phi}_i = \boldsymbol{\psi}_i, \qquad \lambda_i = \omega_i^2 - \mu$$

The matrix iteration procedures are usually used in conjunction with methods derived from the Rayleigh Quotient method. The Rayleigh Quotient method was introduced using distributed

Rayleigh Quotient for Discrete Systems

flexibilty systems and an assumed shape function, but we have seen also an example where the Rayleigh Quotient was computed for a discrete system using an assumed shape vector. The procedure to be used for discrete systems can be summarized as

 $\mathbf{x}(t) = \boldsymbol{\phi} Z_0 \sin \omega t, \qquad \dot{\mathbf{x}}(t) = \omega \boldsymbol{\phi} Z_0 \cos \omega t,$

 $2T_{\max} = \omega^2 \boldsymbol{\phi}^T \mathbf{M} \boldsymbol{\phi},$ $2V_{\text{max}} = \boldsymbol{\phi}^T \mathbf{K} \boldsymbol{\phi},$

equating the maxima, we have

$$\omega^2 = \frac{\boldsymbol{\phi}^T \mathbf{K} \, \boldsymbol{\phi}}{\boldsymbol{\phi}^T \mathbf{M} \, \boldsymbol{\phi}} = \frac{k^*}{m^*}$$

Take note that ϕ is an assumed shape vector, not an eigenvector

Rayleigh Quotient in Ritz Coordinates

We can write the Rayleigh quotient as a function of the Ritz coordinates.

$$\omega^{2}(\mathbf{z}) = \frac{\mathbf{z}^{T} \mathbf{\Phi}^{T} \mathbf{K} \mathbf{\Phi} \mathbf{z}}{\mathbf{z}^{T} \boldsymbol{\phi}^{T} \mathbf{M} \boldsymbol{\phi} \mathbf{z}} = \frac{\overline{k}(\mathbf{z})}{\overline{m}(\mathbf{z})},$$

but this is not an explicit fuction for any modal frequency... On the other hand, we have seen that frequency estimates are always greater than true frequencies, so our best estimates are the the local minima of $\omega^2(\mathbf{z})$, or the points where all the derivatives of $\omega^2(\mathbf{z})$ with respect to z_i are zero:

$$\frac{\partial \omega^2(\mathbf{z})}{\partial z_i} = \frac{\overline{m}(\mathbf{z})\frac{\partial k(\mathbf{z})}{\partial z_i} - \overline{k}(\mathbf{z})\frac{\partial \overline{m}(\mathbf{z})}{\partial z_i}}{(\overline{m}(\mathbf{z}))^2} = 0, \quad \text{for } i = 1, \dots, M < N$$

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Matrix Iteration with Shifts

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The shifted eigenproblem can be solved, e.g., by matrix iteration and the procedure will converge to the *smallest absolute value* shifted eigenvalue and to the associated eigenvector. After convergence is reached

$$\boldsymbol{\psi}_i = \boldsymbol{\phi}_i, \qquad \omega_i^2 = \lambda_i + \mu.$$

The convergence of the method can be greatly enhanced if the shift μ is updated every few steps during the iterative procedure using the current best estimate of λ_i ,

$$\lambda_{i,n+1} = \frac{\hat{\mathbf{x}}_{n+1} \mathbf{M} \, \mathbf{x}_n}{\hat{\mathbf{x}}_{n+1} \mathbf{M} \, \hat{\mathbf{x}}_n},$$

to improve the modified stiffness matrix to be used in the following iterations.

$$\overline{\mathbf{K}} = \overline{\mathbf{K}} - \lambda_{i,n+1} \mathbf{M}$$

Much literature was dedicated to the problem of choosing the initial shifts so that all the eigenvectors can be computed sequentially without missing any of them.

Ritz Coordinates

For a *N DOF* system, an *approximation* to a displacement vector **x** can be written in terms of a set of M < Nassumed shape, linearly independent vectors,

$$\boldsymbol{\phi}_i, \quad i=1,\ldots,M<\Lambda$$

and a set of *Ritz coordinates* z_i , i - 1, ..., M < N:

$$\mathbf{x} = \sum_{i} \boldsymbol{\phi}_{i} Z_{i} = \mathbf{\Phi} \mathbf{z}$$

We say approximation because a linear combination of M < N vectors cannot describe every point in a $N\mbox{-space}.$

Reduced Eigenproblem

Observing that

$$\overline{k}(\mathbf{z}) = \omega^2(\mathbf{z})\overline{m}(\mathbf{z})$$

we can substitute into and simplify the preceding equation,
 $\partial \overline{k}(\mathbf{z}) = \partial \overline{m}(\mathbf{z})$

$$\frac{\partial N(\mathbf{z})}{\partial z_i} - \omega^2(\mathbf{z})\frac{\partial M}{\partial z_i}$$
With the positions

$$\overline{\mathbf{K}} = \mathbf{\Phi}^T \mathbf{K} \mathbf{\Phi}, \quad \overline{\mathbf{M}}$$
$$\overline{\mathbf{K}} = \overline{\mathbf{V}} \mathbf{K} \mathbf{\Phi}, \quad \overline{\mathbf{M}} \mathbf{K} \mathbf{E}$$

$$\overline{k}(\mathbf{z}) = \mathbf{z}^T \overline{\mathbf{K}} \mathbf{z} = \sum_i \sum_j \overline{k}_{ij} Z_j Z_i,$$

$$\frac{\partial \overline{k}(\mathbf{z})}{\partial z_i} = 2 \sum_{i} \overline{k}_{ij} z_j = 2 \overline{\mathbf{K}} \mathbf{z}, \text{ and, analogously, } \frac{\partial \overline{m}(\mathbf{z})}{\partial z_i} = 2 \overline{\mathbf{M}} \mathbf{z}.$$

 $\overline{\mathbf{M}} = \mathbf{\Phi}^T \mathbf{M} \mathbf{\Phi}$

Substituting these results in $\frac{\partial \bar{k}(\mathbf{z})}{\partial z_i} - \omega^2(\mathbf{z}) \frac{\partial \bar{m}(\mathbf{z})}{\partial z_i} = 0$ we can write a *new* homogeneous system in the Ritz coordinates, whose non trivial solutions are the solutions of a reduced eigenvector problem in the MDOF Ritz coordinates space, with reduced $M \times M$ matrices:

 $\overline{\mathbf{K}}\,\mathbf{z}-\omega^2\overline{\mathbf{M}}\,\mathbf{z}=\mathbf{0}.$

Matrix Iteration

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Matrix Iteration with Shifts

Matrix Iteration

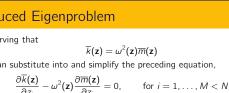
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