Continuous Systems

an example

Giacomo Boffi

http://intranet.dica.polimi.it/people/boffi-giacomo

Dipartimento di Ingegneria Civile Ambientale e Territoriale Politecnico di Milano

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Problem statement



A uniform beam, (unit mass m, flexural stiffness EJ and length L) is loaded by a load P, moving with constant velocity v(t) = v in the time interval $0 \le t \le t_0 = L/v = t_0$. Plot the response in the interval $0 \le t \le t_0 = L/v$ in terms of u(L/2, t) and $M_{\rm b}(L/2, t)$.

NB: the beam is at rest for t = 0.

Equation of motion

F or an uniform beam, the equation of dynamic equilibrium is

$$m \frac{\partial^2 u(x,t)}{\partial t^2} + EJ \frac{\partial^4 u(x,t)}{\partial x^4} = p(x,t).$$

In our example, the loading function must be defined in terms of $\delta(x)$, the Dirac's delta distribution,

$$p(x,t) = P\,\delta(x-vt).$$

The Dirac's delta (or distribution) is defined by

$$\delta(x-x_0) \equiv 0$$
 and $\int f(x)\delta(x-x_0) dx = f(x_0).$

Equation of motion

The solution will be computed by separation of variables

$$u(x,t) = q(t)\phi(x)$$

and modal analysis,

$$u(x,t) = \sum_{n=1}^{\infty} q_n(t)\phi_n(x)$$

The relevant quantities for the modal analysis, obtained solving the eigenvalue problem that arises from the beam boundary conditions are

$$\phi_n(x) = \sin \beta_n x, \qquad \beta_n = \frac{n\pi}{L}, \\ m_n = \frac{mL}{2}, \qquad \omega_n^2 = \beta_n^4 \frac{EJ}{m} = n^4 \pi^4 \frac{EJ}{mL^4}$$

Orthogonality relationships

For an uniform beam, the orthogonality relationships are

$$m \int_{0}^{L} \phi_{n}(x)\phi_{m}(x) dx = m_{n}\delta_{nm},$$

EJ
$$\int_{0}^{L} \phi_{n}(x)\phi_{m}^{\nu}(x) dx = k_{n}\delta_{nm} = m_{n}\omega_{n}^{2}\delta_{nm}$$

(the Kroneker's δ_{nm} is a completely different thing from Dirac's δ , OK?).

Decoupling the EOM

Using the orthogonality relationships, we can write an infinity of uncoupled equation of motion for the modal coordinates.

1 The equation of motion is written in terms of the series representation of u(x, t):

$$m\sum_{m=1}^{\infty}\ddot{q}_{m}\phi_{m}+EJ\sum_{m=1}^{\infty}q_{m}\phi_{m}^{\mathsf{iv}}=P\,\delta(x-vt),$$

2 every term is multiplied by ϕ_n and integrated over the lenght of the beam

$$m\int_0^L \phi_n \sum_1^\infty \ddot{q}_m \phi_m \, \mathrm{d}x + EJ \int_0^L \phi_n \sum_1^\infty q_m \phi_m^{\mathsf{iv}} \, \mathrm{d}x = P \int_0^L \phi_n \delta(x - vt), \, n = 1, \dots, \infty$$

3 we use the ortogonality relationships and the definition of δ ,

$$m_n\ddot{q}(t) + k_nq(t) = P\phi_n(vt) = P\sin\frac{n\pi\,vt}{L}, \qquad n = 1, \dots, \infty.$$

Solutions

Considering that

- the initial conditions are zero for all the modal equations,
- for each mode we have a *different* excitation frequency $\overline{\omega}_n = n\pi v/L$ (and also $\beta_n = \overline{\omega}_n / \omega_n$),

the individual solutions are given by

$$q_n(t) = \frac{P}{k_n} \frac{1}{1 - \beta_n^2} \left(\sin \overline{\omega}_n t - \beta_n \sin \omega_n t \right), \quad 0 \le t \le \frac{L}{v}$$

and, with $k_n=m_n\omega_n^2=rac{mL}{2}\;n^4\pi^4rac{EJ}{mL^4}=n^4\pi^4rac{EJ}{2L^3}$, it is

$$q_n(t) = \frac{2}{n^4 \pi^4} \frac{PL^3}{EJ} \frac{1}{1 - \beta_n^2} \left(\sin \overline{\omega}_n t - \beta_n \sin \omega_n t \right), \quad 0 \le t \le \frac{L}{v}.$$

It is apparent that we have *resonance* for $\beta_n = 1$.

Critical Velocity

Let's start from $\beta_1 = \pi v/L/\omega_1 = 1$ and solve for the velocity, say v_1

$$v_1 = \omega_1 L / \pi.$$

It is apparent that v_1 is a critical velocity $v_c = v_1 = \omega_1 L/\pi$ that gives a resonance condition for the first mode response, while for $v = 2 v_c$ the second mode is in resonance, etc.

With the position $v = \kappa v_1$ it is

$$\overline{\omega}_n = \kappa n \omega_1$$
 and $\beta_n = n \kappa \omega_1 / n^2 \omega_1 = \kappa / n$

and we can rewrite the solution as

$$q_n(t) = \frac{2PL^3}{\pi^4 EJ} \frac{1}{n^2(n^2 - \kappa^2)} \left(\sin(\frac{\kappa}{n}\omega_n t) - \frac{\kappa}{n} \sin\omega_n t \right), \quad 0 \le t \le \frac{L}{\nu}.$$

Adimensional Time Coordinate

Introducing an adimensional time coordinate ξ with $t=t_0\xi$, noting that $\omega_n=n^2\omega_1$ we can write

$$\frac{\kappa}{n}\omega_n t = \frac{\kappa}{n}n^2\omega_1\,\xi\,t_0 = \kappa n(\frac{v_c\pi}{L})\xi\frac{L}{\kappa v_c} = n\pi\xi,$$

substituting in the solution for mode n we have

$$q_n(\xi) = \frac{2}{\pi^4} \frac{PL^3}{EJ} \frac{1}{n^2(n^2 - \kappa^2)} \left(\sin(n\pi\xi) - \frac{\kappa}{n} \sin(\frac{n^2}{\kappa}\pi\xi) \right), \qquad 0 \le \xi \le 1.$$

Adimensional Time and Adimensional Position

If we denote with X(t) the position of the load at time t, it is $X(t) = vt = \xi L$, or $\xi = X/L$ and the expression $u(x,\xi) = \sum q_n(\xi)\phi_n(x)$ can be interpreted as the displacement in x when the load is positioned in ξL .

Displacement and Bending Moment

The displacement and the bending moment are given by

$$u(x,\xi) = \frac{2PL^3}{\pi^4 EJ} \sum_{n=1}^{\infty} \frac{1}{n^2(n^2 - \kappa^2)} \left(\sin(n\pi\xi) - \frac{\kappa}{n} \sin(\frac{n^2}{\kappa}\pi\xi) \right) \sin(n\pi\frac{x}{L}),$$

$$M_{\rm b}(x,\xi) = -EJ \frac{\partial^2 u(x,\xi)}{\partial x^2}$$
$$= \frac{2PL}{\pi^2} \sum_{n=1}^{\infty} \frac{1}{n^2 - \kappa^2} \left(\sin(n\pi\xi) - \frac{\kappa}{n} \sin(\frac{n^2}{\kappa}\pi\xi) \right) \sin(n\pi\frac{x}{L}).$$

Normalized Midspan Deflection

If we consider the midspan deflection (bending moment) due to a static load P on the beam, the maximum deflection (bending moment) is expected when the load is placed at midspan, and it is

$$u_{\text{stat}}(L/2, 1/2) = \frac{PL^3}{48EJ}$$
 and $M_{\text{b stat}}(L/2, 1/2) = \frac{PL}{4}$.

Normalizing the midspan displacement with respect to the maximum static displacement, we write

$$\Delta(\xi) = \frac{u}{u_{\text{stat}}} = \frac{96}{\pi^4} \sum_{n=1}^{\infty} \frac{1}{n^2 (n^2 - \kappa^2)} \left(\sin(n\pi\xi) - \frac{\kappa}{n} \sin(\frac{n^2}{\kappa}\pi\xi) \right) \frac{\sin(n\pi\xi)}{2}.$$

Eventually we introduce a notation for the partial sum of the first N terms:

$$\Delta_N(\xi) = \frac{96}{\pi^4} \sum_{n=1}^N \frac{1}{n^2(n^2 - \kappa^2)} \left(\sin(n\pi\xi) - \frac{\kappa}{n} \sin(\frac{n^2}{\kappa}\pi\xi) \right) \frac{1}{\sin(n\pi\xi)} \frac{1}{2} \frac{1}{2$$

Normalized Midspan Bending Moment

Analogously, normalizing with respect to the maximum static bending moment, it is

$$\mu(\xi) = \frac{8}{\pi^2} \sum_{n=1}^{\infty} \frac{1}{n^2 - \kappa^2} \left(\sin(n\pi\xi) - \frac{\kappa}{n} \sin(\frac{n^2}{\kappa}\pi\xi) \right) \sin(n\frac{\pi}{2}),$$

the partial sum being denoted by

$$\mu_N(\xi) = \frac{8}{\pi^2} \sum_{n=1}^N \frac{1}{n^2 - \kappa^2} \left(\sin(n\pi\xi) - \frac{\kappa}{n} \sin(\frac{n^2}{\kappa}\pi\xi) \right) \sin(n\frac{\pi}{2}).$$

Error Estimates

To appreciate the approximation inherent in a truncated series, we compare the truncated series computed for $\kappa = 10^{-6}$ with the static response $\Delta_{\rm stat}(\xi) = 3\xi - 4\xi^3$ introducing a percent error function

$$\epsilon_{u,N}(\xi) = 100 \left(1 - \frac{\Delta_N(\xi)|_{\kappa = 10^{-6}}}{\Delta_{\text{stat}}(\xi)} \right) \quad \text{for } 0 \le \xi \le 1/2,$$



Using 4 terms (N = 7) the absolute error is not greater than 1/1000.

Error Estimates

Analogously we can use the midspan bending moment, normalized with respect to PL/4, $\mu_{\text{stat}}(\xi) = 2\xi$ to define another percent error function

$$\epsilon_{M,N} = 100 \left(1 - \frac{\mu_N(\xi)|_{\kappa=10^{-6}}}{\mu_{\text{stat}}(\xi)} \right)$$



The Plots

Eventually, we plot the normalized displacement and the normalized bending moment for different values of κ , i.e., for different velocities.

For the displacement I used N = 11 while for the bending moment I used N = 25.



